



Application for Conditional Use Permit

Applicant Name: _____

Address: _____ City: _____

State: _____ Zip: _____

Telephone: _____ Cell Phone: _____

Email: _____

Agent Name: _____

Address: _____ City: _____

State: _____ Zip: _____

Telephone: _____ Cell Phone: _____

Email: _____

Property Address (Es): _____

Current Zoning: _____

Current/Most Recent Property Use: _____

Proposed Use: _____





The application will be evaluated using the standards of Sec. 114-154 of the Municipal Code (below). Please use the space to justify and explain how your proposal addresses these conditions; use an additional sheet if necessary.

- (1) The establishment, maintenance, or operation of the conditional use will not be detrimental to, or endanger, the public health, safety, morals, comfort, or general welfare;

- (2) The conditional use will not be injurious to the use and enjoyment of other property in the immediate vicinity for the purposes already permitted, nor substantially diminish and impair property values within the neighborhood;

- (3) The establishment of the conditional use will not impede the normal and orderly development and improvement of the surrounding property for uses permitted in the district;

- (4) Adequate utilities, access roads, drainage and/or necessary facilities have been or are being provided;

- (5) Adequate measures have been or will be taken to provide ingress and egress so designed as to minimize traffic congestion in the public streets;

- (6) The proposed conditional use is not contrary to the objectives of the current land use plan for the city; and

- (7) The conditional use shall, in all other respects, conform to the applicable regulations of the district in which it is located, except as such regulations may, in each instance, be modified pursuant to the recommendations of the plan commission.





If the required supplemental materials, which constitute a completed application, are not submitted, the application will not be processed.

Required Submittal Format

1. An electronic submission via email/USB drive/CD/Download link; and
2. One (1) paper copy, no larger than 11" x 17" size.

Required Submittal Item	Applicant Submitted	City Received
1. Conditional Use Review Application		
2. Written description of project, including: a. Hours of operation b. Anticipated delivery schedule c. Maintenance plan d. General use of the building and lot		
3. Site Plan (drawn to scale), including: a. Fully dimensioned property boundary b. All buildings (existing and proposed) c. Setbacks from property lines d. Identification as to whether all elements are "Existing" or "Proposed" e. Dimensioned parking spaces and drive aisle layout f. Trash enclosure location and materials g. Loading spaces h. Fire hydrant locations i. Location of signage, with setbacks		
4. Zoning Analysis Table a. Land area (in acres and square feet) b. Building area (in square feet) c. Setbacks (required yards in feet) d. Floor Area Ratio (building area divided by lot area) e. Lot Coverage (building footprint divided by lot area) f. Height of all buildings and structures g. Percentage of greenspace (landscaped areas divided by lot area) h. Parking spaces		
5. Landscape Plan a. Bufferyards b. Parking Areas c. Screening and fencing locations d. Plant lists including the following: Latin and Common Names, Number of each planting material, and Size at planting.		





DEPARTMENT OF CITY DEVELOPMENT



Required Submittal Item	Applicant Submitted	City Received
6. Lighting Plan a. Location of light fixtures b. A cut sheet of light fixtures with indication of cut-offs or shielding c. Illumination diagram indicating intensity of lighting on the property.	☒	
7. Floor Plan a. Preliminary floor plan layout of all buildings/structures b. Labels for the type of use of the area c. Labels for square footage of the area	☒	
8. Engineering Plan a. Stormwater Plan (Drainage pattern, flow, detention) b. Existing and proposed roadway and access configurations c. Cross access	☒	
9. Signage Plan a. dimensioned color elevations of signage b. A diagram showing the location of the proposed signage	☒	
10. Building/site elevations (if new building or exterior changes planned) a. Building elevations showing all four sides of the buildings in color b. Elevation of trash enclosure area	☒	
11. Building Material Samples (if making exterior changes)	☒	
12. Review Fee	☒	

Acknowledgement and authorization signatures

A conditional use is not like a building permit; applying does not mean it will be approved.

The approval may contain conditions related to the improvement of the site which must be met prior to the issuance of a building occupancy permit. Conditions related to the operational aspect(s) of the business must be complied with at all times. That, in the event site improvement work required by ordinance cannot be completed prior to desired occupancy, a financial assurance, at 100% of the improvement estimate, guaranteeing completion of the required improvements must be placed on file with the City of Racine. Estimates and Assurance documents are subject to the review and final approval by the City. Improvements may include but are not limited to landscaping, fencing, lighting, pavement surfacing and sealing, dumpster enclosures, and exterior building improvements;

The signature(s) hereby certify that the statements made by myself and constituting part of this application are true and correct. I am fully aware that any misrepresentation of any information on this application may be grounds for denial of this application.

Owner Signature (acknowledgement and authorization):

Date: 8/18/22

Applicant Signature (acknowledgement):

Joe Goldshlack

Digitally signed by Joe Goldshlack
DN: cn=Joe Goldshlack, o=Fullerton Engineering, ou=Zoning & Permitting, email=jgoldshlack@fullertonengineering.com, c=US
Date: 2022.07.18 16:39:31 -0500

Date: July 18, 2022





DISH Wireless L.L.C. SITE ID:
MWMKE00116B

DISH Wireless L.L.C. SITE ADDRESS:
**314 6TH STREET
RACINE, WI 53403**

WISCONSIN CODE COMPLIANCE

ALL WORK SHALL BE PERFORMED AND MATERIALS INSTALLED IN ACCORDANCE WITH THE CURRENT EDITIONS OF THE FOLLOWING CODES AS ADOPTED BY THE LOCAL GOVERNING AUTHORITIES. NOTHING IN THESE PLANS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES:

CODE TYPE	CODE
BUILDING	WISCONSIN CBC/2015 IBC W/ WI AMENDMENTS
MECHANICAL	WISCONSIN CBC/2015 IMC W/ WI AMENDMENTS
ELECTRICAL	WISCONSIN ELECTRICAL CODE/2017 NEC W/ WI AMENDMENTS

SHEET INDEX

SHEET NO.	SHEET TITLE
T-1	TITLE SHEET
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A-2	ENLARGED BUILDING PLAN
A-2A	ENLARGED EQUIPMENT PLAN
A-3	ANTENNA PLAN AND SCHEDULE
A-4	BUILDING ELEVATION
A-5	EQUIPMENT PLATFORM AND H-FRAME DETAILS
A-6	EQUIPMENT DETAILS
A-7	EQUIPMENT DETAILS
E-1	UTILITY RISER DIAGRAM
E-2	ELECTRICAL DETAILS
E-3	ELECTRICAL ONE-LINE AND PANEL SCHEDULE
G-1	GROUNDING PLANS AND NOTES
G-2	GROUNDING DETAILS
G-3	GROUNDING DETAILS
RF-1	RF CABLE COLOR CODE
GN-1	LEGEND AND ABBREVIATIONS
GN-2	RF SIGNAGE
GN-3	GENERAL NOTES
GN-4	GENERAL NOTES
GN-5	GENERAL NOTES
S-1	STRUCTURAL NOTES
S-2	STRUCTURAL DETAILS
S-3	STRUCTURAL DETAILS
S-4	MOUNTING SPECIFICATIONS

SCOPE OF WORK

THIS IS NOT AN ALL INCLUSIVE LIST. CONTRACTOR SHALL UTILIZE SPECIFIED EQUIPMENT PART OR ENGINEER APPROVED EQUIVALENT. CONTRACTOR SHALL VERIFY ALL NEEDED EQUIPMENT TO PROVIDE A FUNCTIONAL SITE. THE PROJECT GENERALLY CONSISTS OF THE FOLLOWING:

SECTOR SCOPE OF WORK:

- INSTALL (3) PROPOSED PANEL ANTENNAS (1 PER SECTOR)
- INSTALL (3) PROPOSED ANTENNA MOUNTS (1 PER SECTOR)
- INSTALL PROPOSED JUMPERS
- INSTALL (6) PROPOSED RRUs (2 PER SECTOR)
- INSTALL (3) PROPOSED OVER VOLTAGE PROTECTION DEVICE (OVP) (1 PER SECTOR)
- INSTALL (3) PROPOSED HYBRID CABLES
- INSTALL PROPOSED CABLE TRAY

ROOFTOP SCOPE OF WORK:

- INSTALL (1) PROPOSED EQUIPMENT CABINET AND EQUIPMENT PLATFORM ON PROPOSED STEEL SUPPORT BEAMS BETWEEN PARAPET WALL AND ELEVATED ROOF
- INSTALL (1) H-FRAME
- INSTALL (1) PROPOSED BBU IN CABINET
- INSTALL (1) PROPOSED POWER CONDUIT AND (1) GROUNDING CONDUIT
- INSTALL (1) PROPOSED TELCO CONDUIT
- INSTALL (1) PROPOSED PPC CABINET
- INSTALL (1) PROPOSED NEMA 3 TELCO-FIBER BOX
- INSTALL (1) PROPOSED GPS UNIT
- INSTALL (1) METER SOCKET ON BUILDING WALL

SITE PHOTO



UNDERGROUND SERVICE ALERT - DIGGERS HOTLINE
UTILITY NOTIFICATION CENTER OF WISCONSIN
(800) 242-8511
WWW.DIGGERSHOTLINE.COM



CALL 3 WORKING DAYS UTILITY NOTIFICATION PRIOR TO CONSTRUCTION

GENERAL NOTES

THE FACILITY IS UNMANNED AND NOT FOR HUMAN HABITATION. A TECHNICIAN WILL VISIT THE SITE AS REQUIRED FOR ROUTINE MAINTENANCE. THE PROJECT WILL NOT RESULT IN ANY SIGNIFICANT DISTURBANCE OR EFFECT ON DRAINAGE. NO SANITARY SEWER SERVICE, POTABLE WATER, OR TRASH DISPOSAL IS REQUIRED AND NO COMMERCIAL SIGNAGE IS PROPOSED.

11"x17" PLOT WILL BE HALF SCALE UNLESS OTHERWISE NOTED

CONTRACTOR SHALL VERIFY ALL PLANS, EXISTING DIMENSIONS, AND CONDITIONS ON THE JOB SITE, AND SHALL IMMEDIATELY NOTIFY THE ENGINEER IN WRITING OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.

SITE INFORMATION

PROPERTY OWNER: C AND A HOME AND BUILDING LLC
ADDRESS: 901 COLLEGE AVE.
RACINE, WI 53403

STRUCTURE TYPE: ROOFTOP

COUNTY: RACINE

LATITUDE (NAD 83): 42° 43' 37.14" N
42.726983°

LONGITUDE (NAD 83): 87° 47' 4.79" W
-87.784664°

ZONING JURISDICTION: CITY OF RACINE

ZONING DISTRICT: AR

PARCEL NUMBER: 276000000286000

OCCUPANCY GROUP: U

CONSTRUCTION TYPE: II-B

POWER COMPANY: WE ENERGIES

TELEPHONE COMPANY: TBD

PROJECT DIRECTORY

APPLICANT: DISH Wireless, LLC.
5701 SOUTH SANTA FE DRIVE
LITTLETON, CO 80120

SITE DESIGNER: FULLERTON ENGINEERING
1100 E WOODFIELD RD., STE 500
SCHAUMBURG, IL 60173
(847) 908-8400

SITE ACQUISITION: MEGHAN LANIGAN

CONSTRUCTION MANAGER: ANDREW HILGERSON

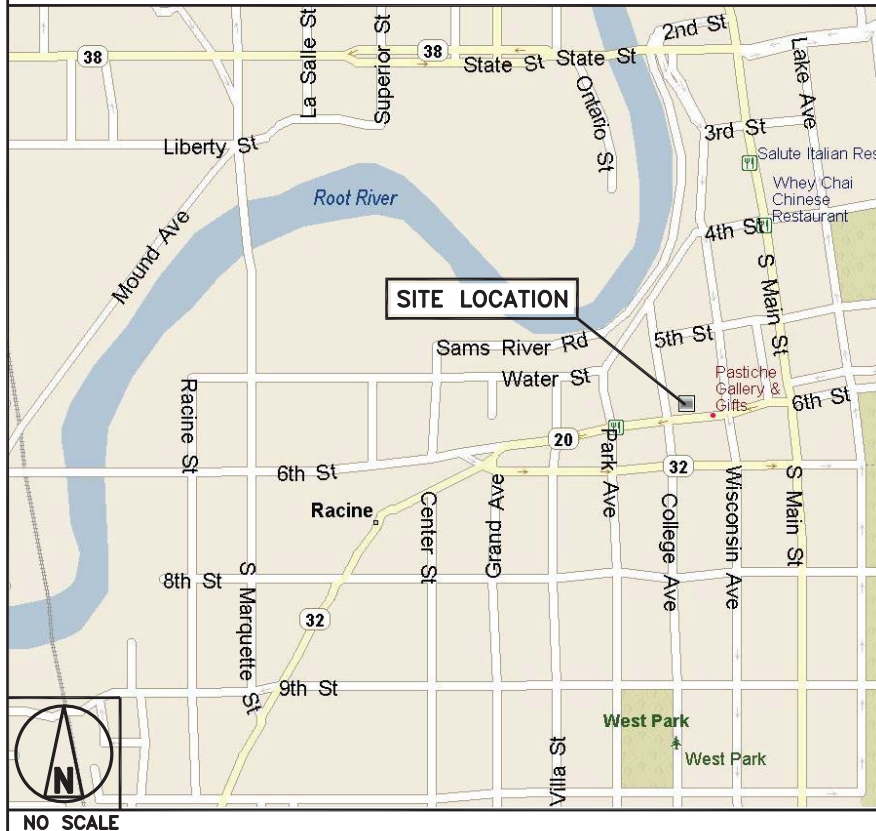
RF ENGINEER: INGA BRAUNEIS

DIRECTIONS

DIRECTIONS FROM JOHN H BATTEN MEMORIAL AIRPORT:

TURN (LEFT) ONTO WI-38 [NORTHWESTERN AVE].
TURN LEFT (EAST) ONTO WI-38 [STATE ST].
TURN RIGHT (SOUTH) ONTO WISCONSIN AVE.
TURN RIGHT (WEST) ONTO WI-20 [W-32].
TURN RIGHT (NORTH) ONTO LOCAL ROAD(S).
ARRIVE AT SITE.

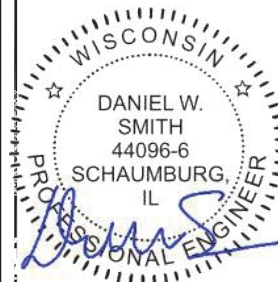
VICINITY MAP



5701 SOUTH SANTA FE DRIVE
LITTLETON, CO 80120



1100 E. WOODFIELD ROAD, SUITE 500
SCHAUMBURG, ILLINOIS 60173
TEL: 847-908-8400
COA# 3620-11
www.FullertonEngineering.com



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DRAWN BY: CHECKED BY: APPROVED BY:

LA DZ DS

RFDS REV #: 2

PERMITTING DOCUMENTS

SUBMITTALS		
REV	DATE	DESCRIPTION
A	03/31/2022	ISSUED FOR REVIEW
B	05/11/2022	ISSUED FOR REVIEW
O	07/07/2022	ISSUED FOR PERMITTING

A&E PROJECT NUMBER
2021.0030.0004

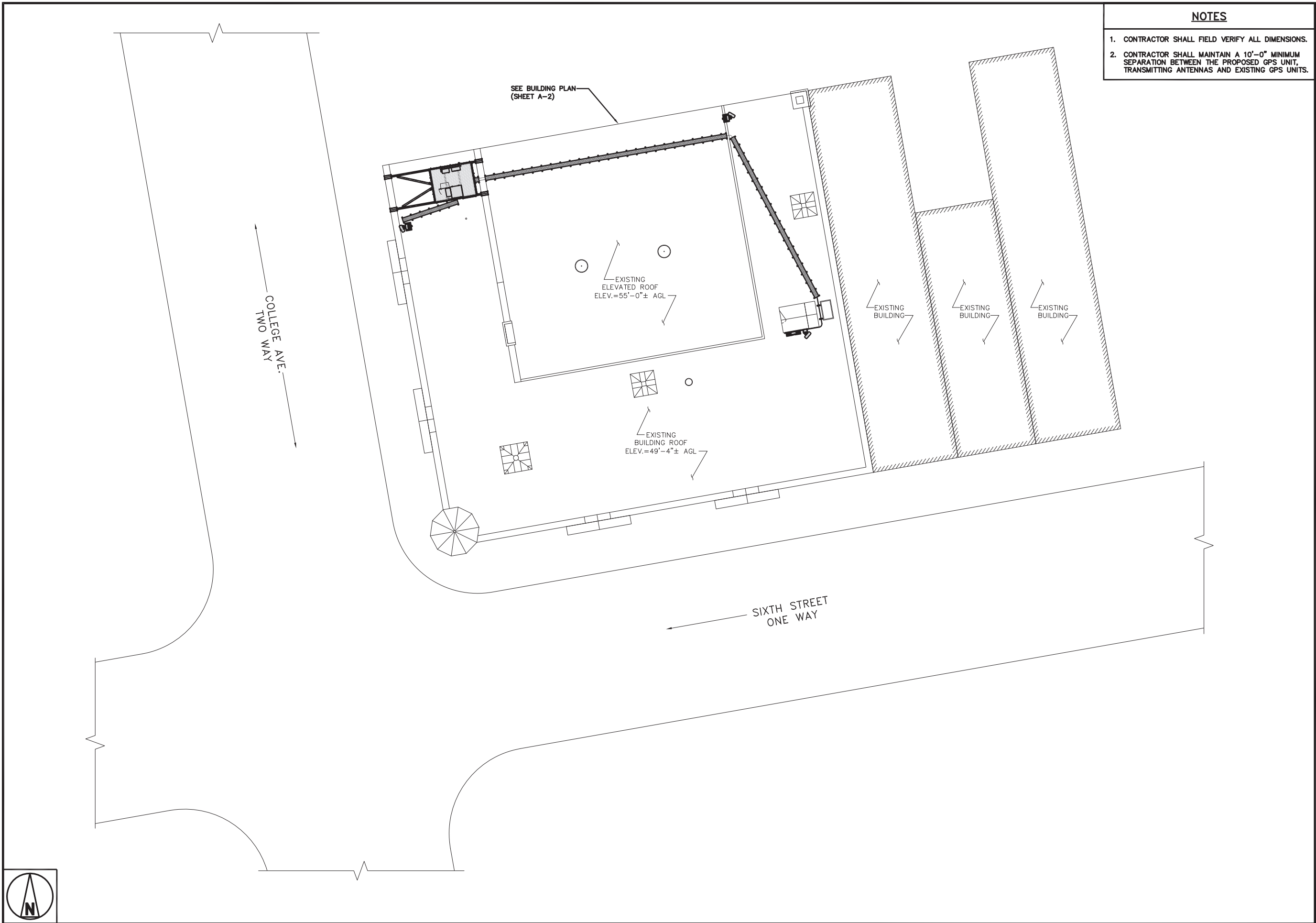
DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
TITLE SHEET

SHEET NUMBER

T-1



NOTES

1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS.
2. CONTRACTOR SHALL MAINTAIN A 10'-0" MINIMUM SEPARATION BETWEEN THE PROPOSED GPS UNIT, TRANSMITTING ANTENNAS AND EXISTING GPS UNITS.



5701 SOUTH SANTA FE DRIVE
LITTLETON, CO 80120



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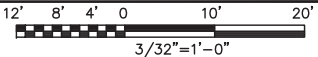
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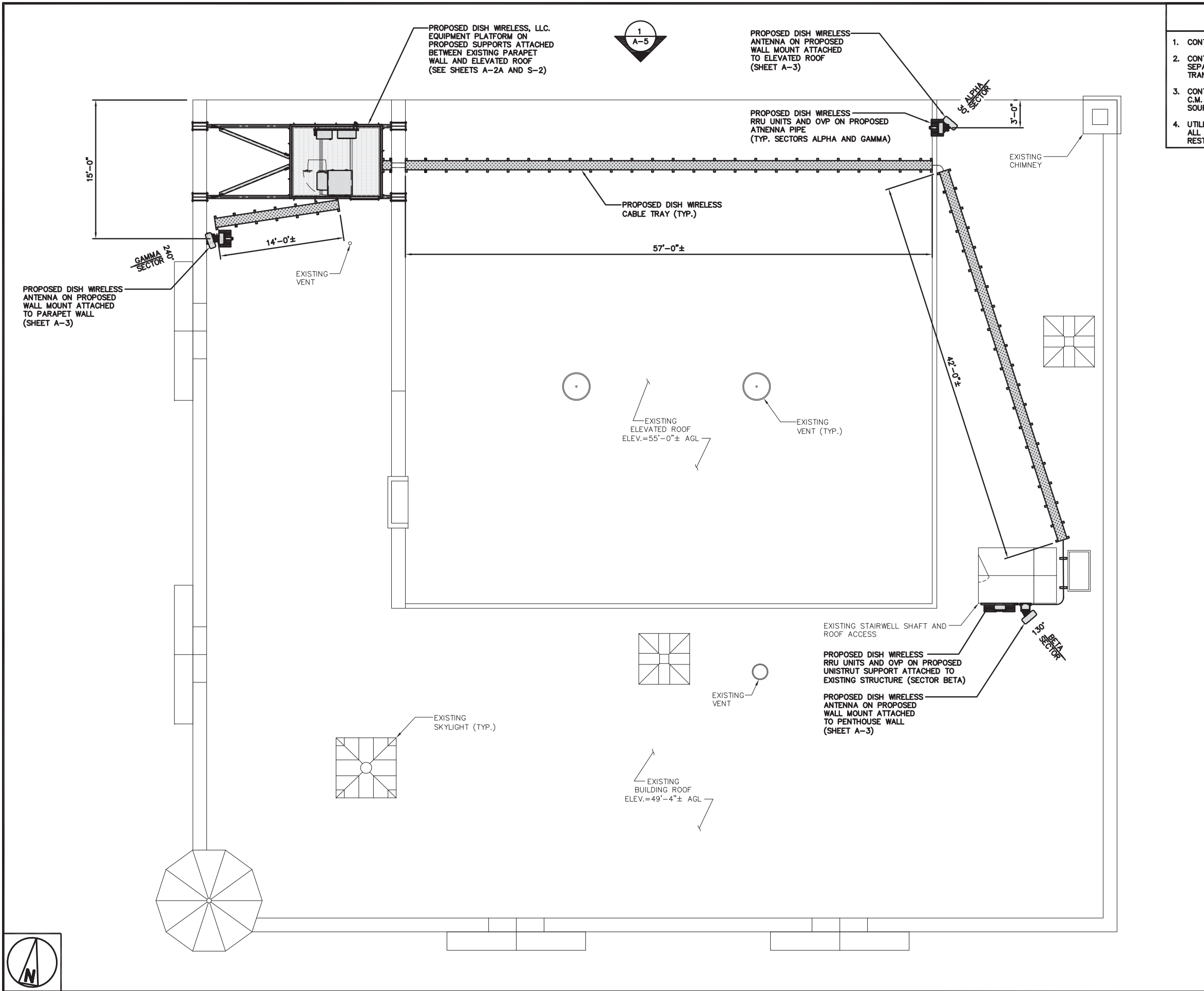
SHEET TITLE
OVERALL
SITE PLAN

SHEET NUMBER

A-1

OVERALL SITE PLAN





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- 3. CONTRACTOR TO VERIFY WITH DISH Wireless L.L.C. C.M. THE LOCATION OF THE POWER AND FIBER SOURCE PRIOR TO CONSTRUCTION.
- 4. UTILITY RUBBER MAT TO BE IN STALLED UNDER ALL DISH Wireless L.L.C. EQUIPMENT THAT IS RESTING ON OR AFFIXED TO ROOF MEMBRANE



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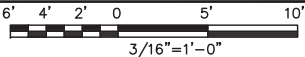
DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
ENLARGED
BUILDING PLAN

SHEET NUMBER
A-2

ENLARGED BUILDING PLAN





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LITTLETON, CO 80120



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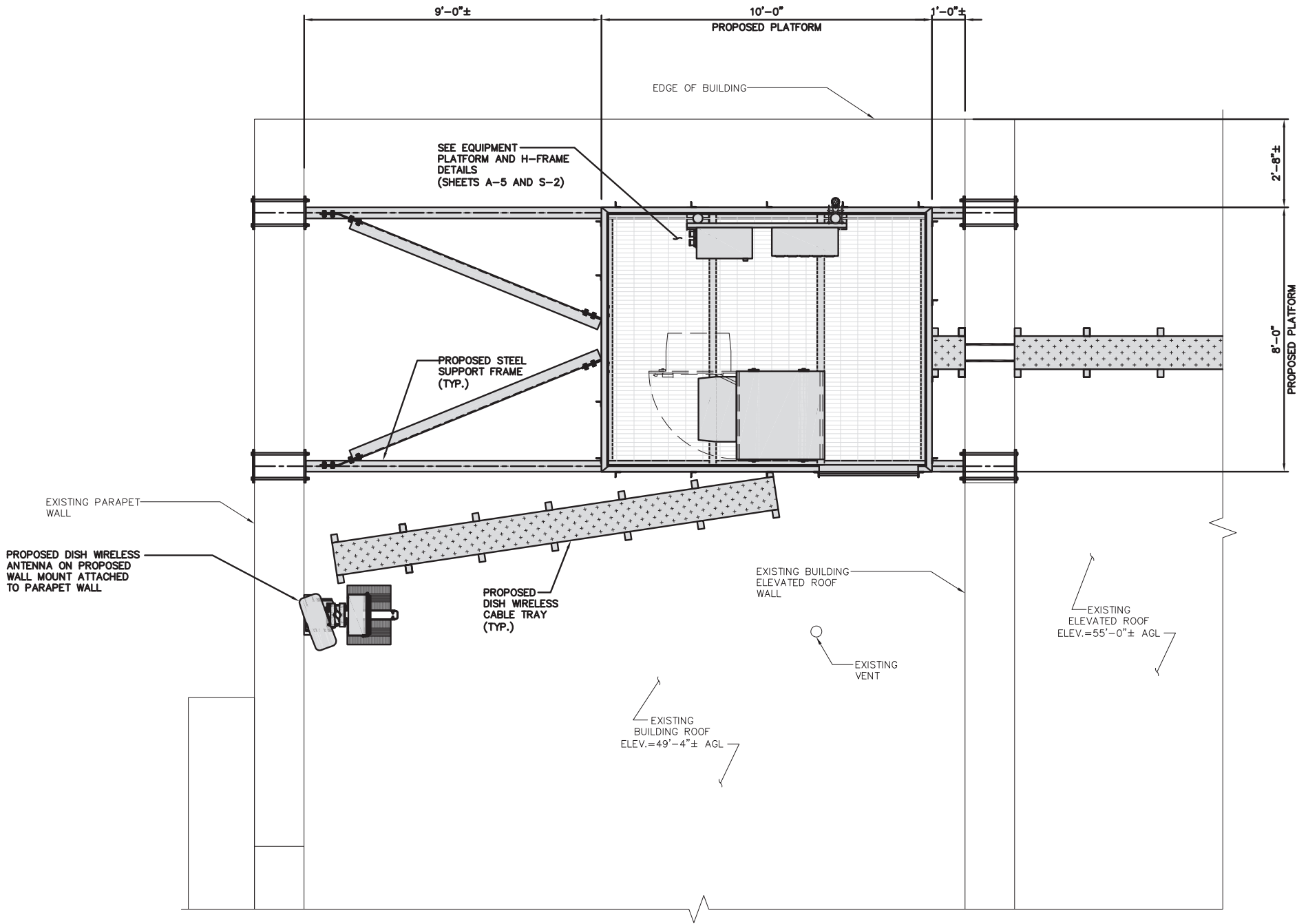
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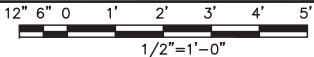
SHEET TITLE
ENLARGED
EQUIPMENT PLAN

SHEET NUMBER

A-2A



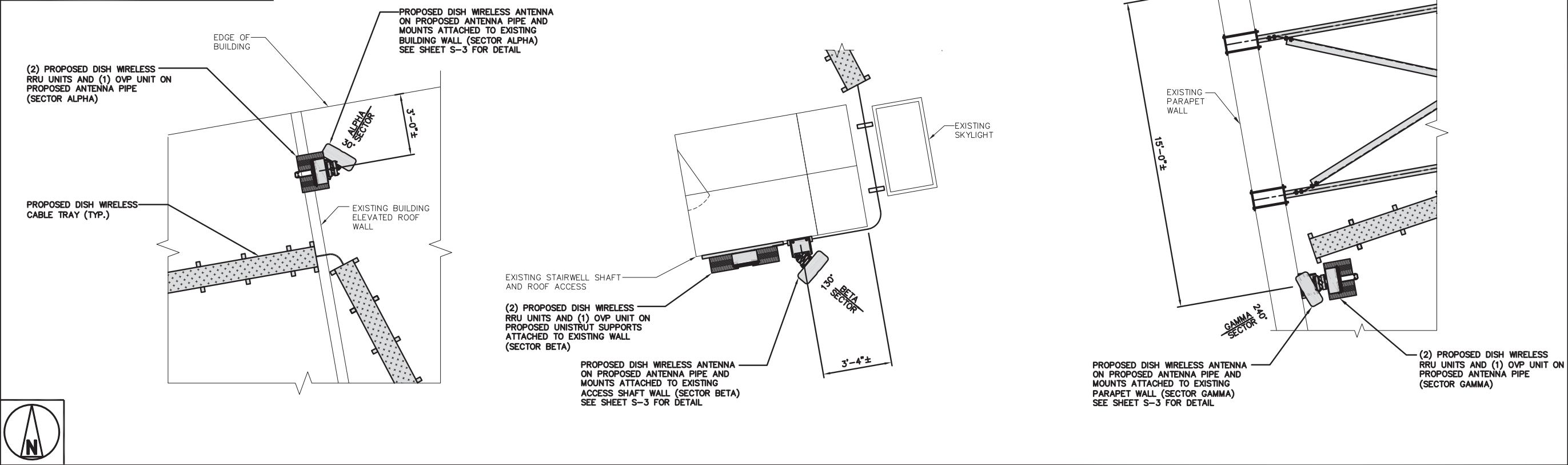
ENLARGED EQUIPMENT PLAN



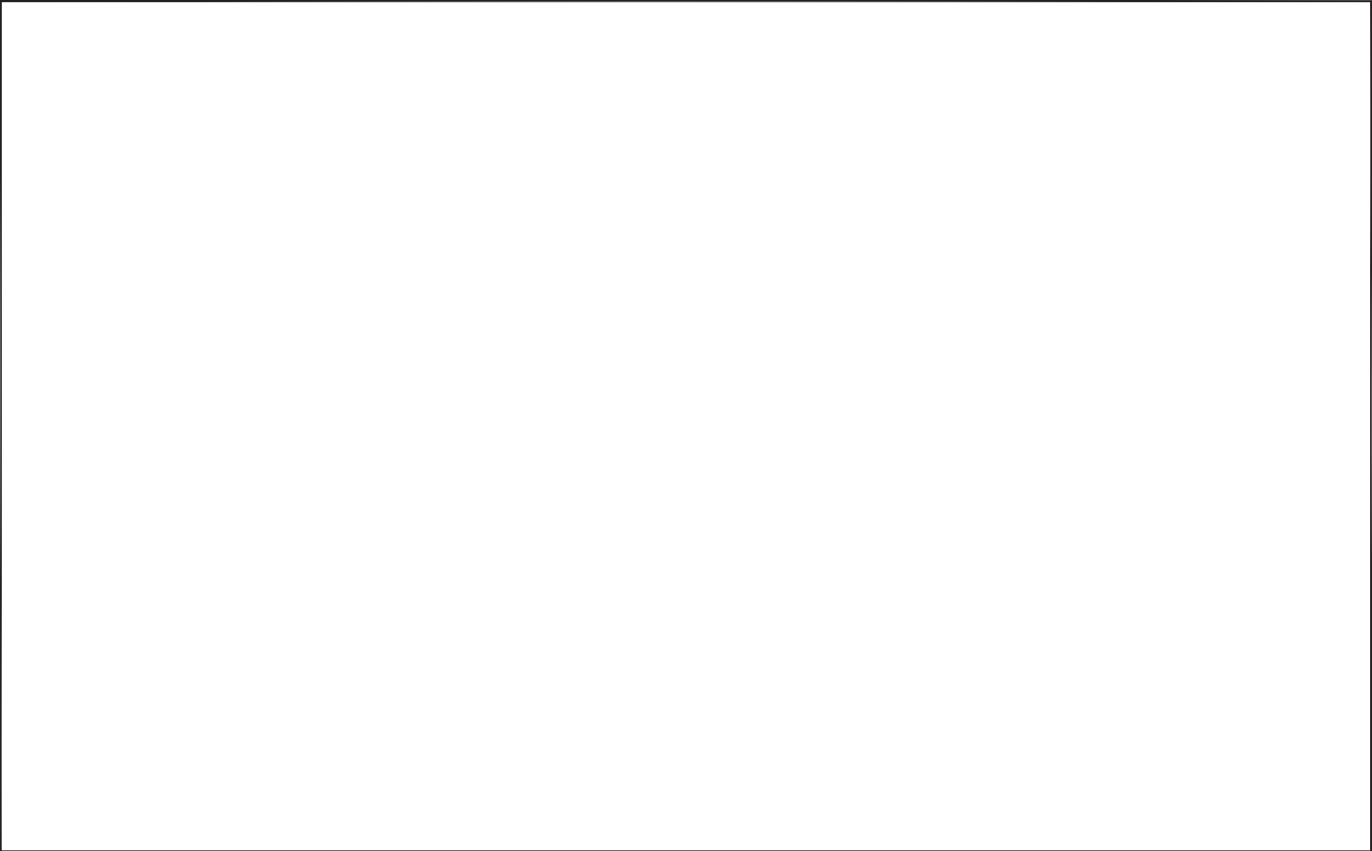
NOTES

1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS.

2. ANTENNA SPECIFICATIONS – REFER TO ANTENNA SCHEDULE AND TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS



ANTENNA PLAN

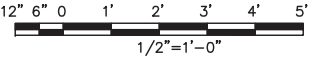


SECTOR	POSITION	ANTENNA						TRANSMISSION CABLE
		EXISTING OR PROPOSED	MANUFACTURER – MODEL NUMBER	TECHNOLOGY	SIZE (HxW)	AZIMUTH	RAD CENTER	FEED LINE TYPE AND LENGTH
ALPHA	A1	PROPOSED	COMMSCOPE FFVV-65B-R2	5G	72.0" x 19.6"	30°	59'-0"	(1) HIGH-CAPACITY HYBRID CABLE (75'±)
BETA	B1	PROPOSED	COMMSCOPE FFVV-65B-R2	5G	72.0" x 19.6"	130°	59'-0"	(1) HIGH-CAPACITY HYBRID CABLE (125'±)
GAMMA	C1	PROPOSED	COMMSCOPE FFVV-65B-R2	5G	72.0" x 19.6"	240°	59'-0"	(1) HIGH-CAPACITY HYBRID CABLE (25'±)
SECTOR	POSITION	RRH		NOTES				
		MANUFACTURER – MODEL NUMBER	TECHNOLOGY					
ALPHA	A1	FUJITSU – TA08025-B604	5G					
	A1	FUJITSU – TA08025-B605	5G					
BETA	B1	FUJITSU – TA08025-B604	5G					
	B1	FUJITSU – TA08025-B605	5G					
GAMMA	C1	FUJITSU – TA08025-B604	5G					
	C1	FUJITSU – TA08025-B605	5G					

1. CONTRACTOR TO REFER TO FINAL CONSTRUCTION RFDS FOR ALL RF DETAILS.

2. ANTENNA AND RRH MODELS MAY CHANGE DUE TO EQUIPMENT AVAILABILITY. ALL EQUIPMENT CHANGES MUST BE APPROVED AND REMAIN IN COMPLIANCE WITH THE PROPOSED DESIGN AND STRUCTURAL ANALYSES.

NOT USED



2

ANTENNA SCHEDULE

NO SCALE

3



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RFDS REV #: 2

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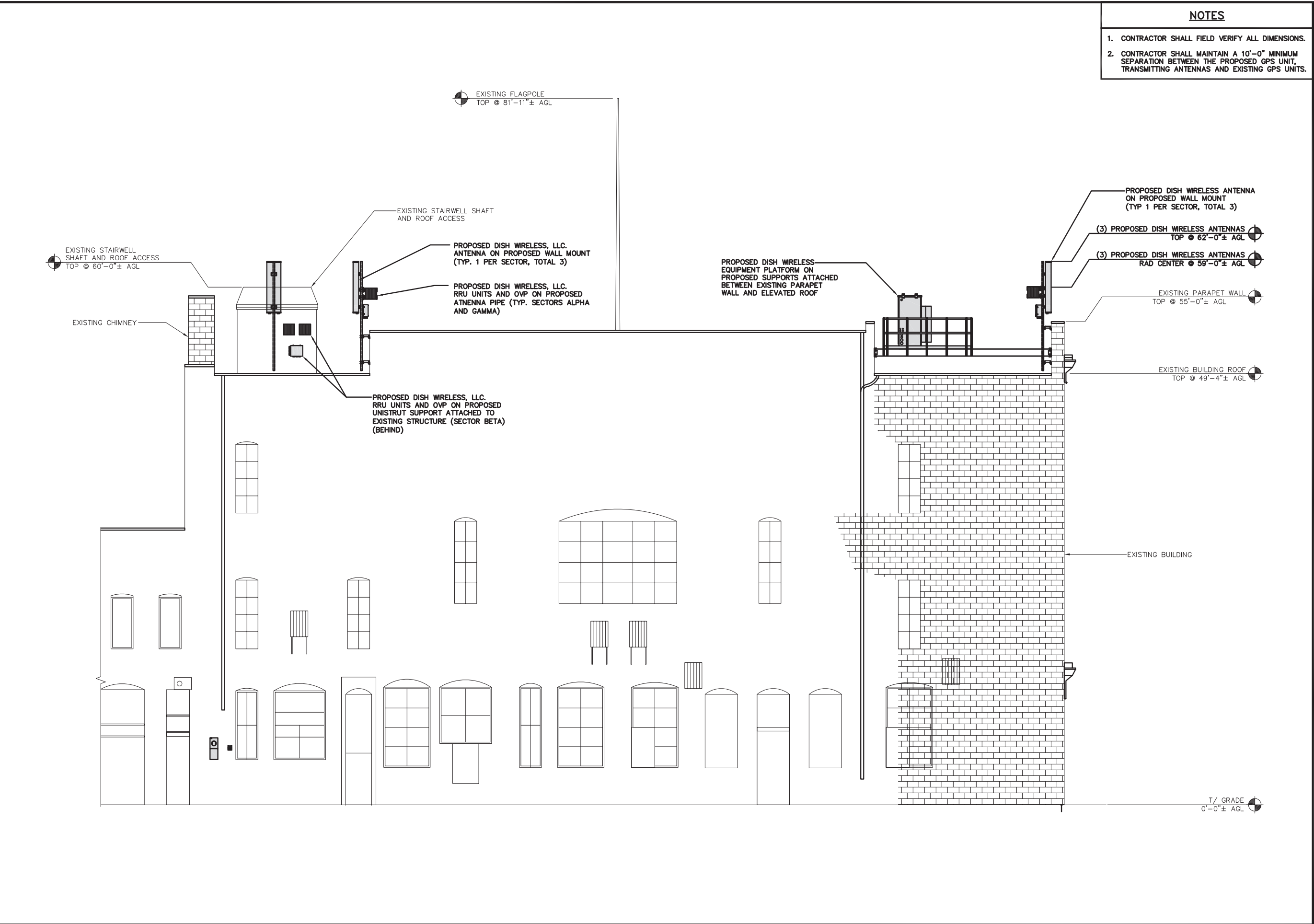
DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
ANTENNA PLAN,
ELEVATION AND SCHEDULE

SHEET NUMBER

A-3



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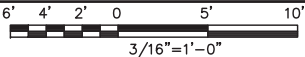
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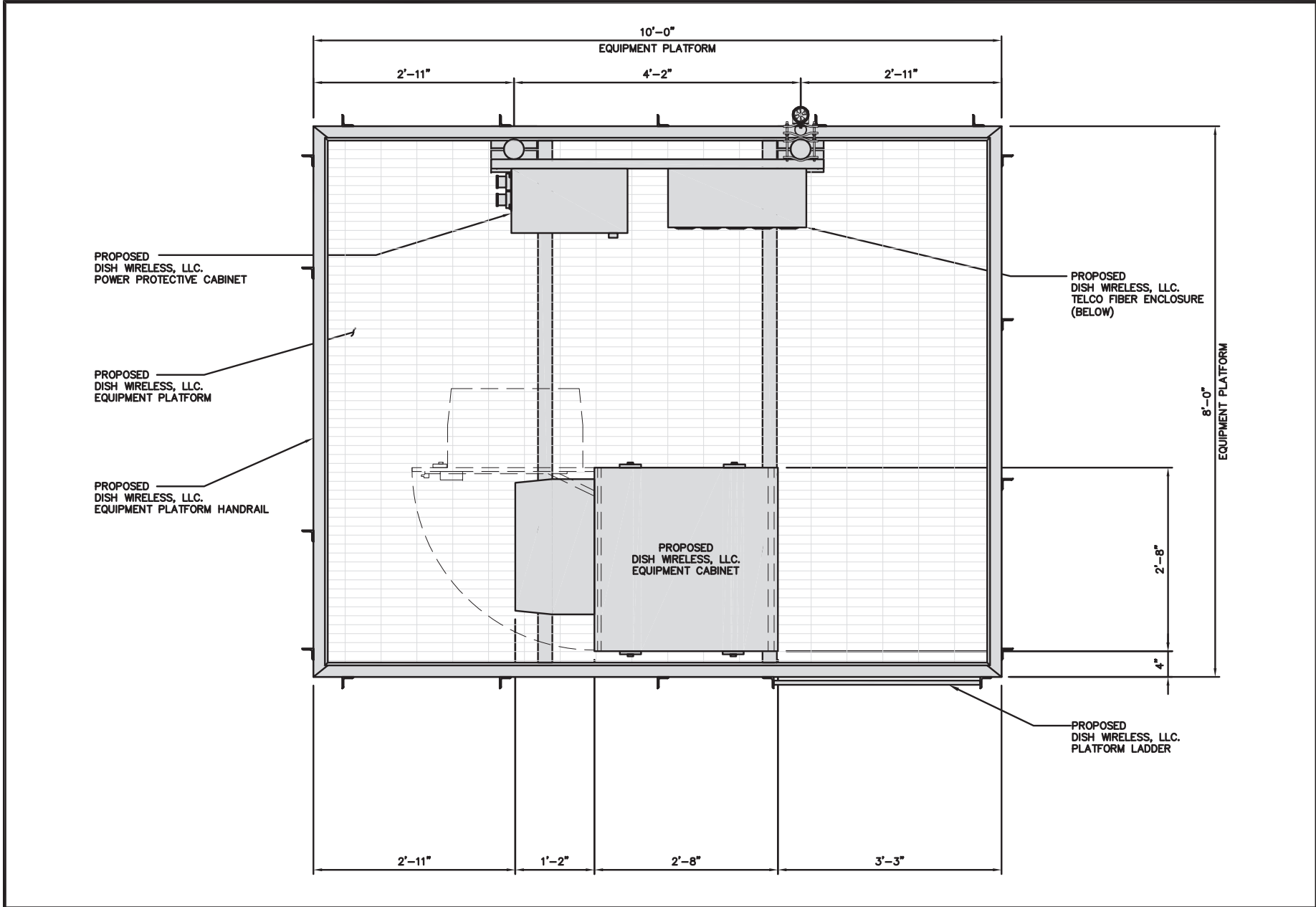
SHEET TITLE
BUILDING
ELEVATION

SHEET NUMBER

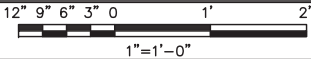
A-4

BUILDING NORTH ELEVATION





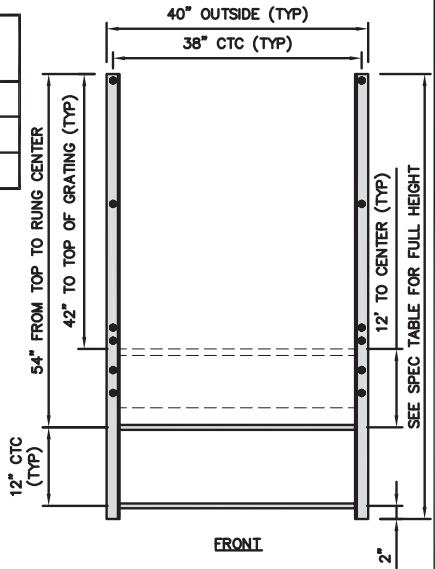
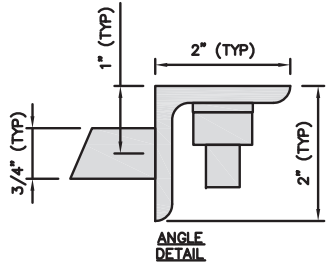
PLATFORM EQUIPMENT PLAN



1

EQL.2 PLATFORM ACCESS LADDER	
RUNG WELDMENT QTY	2
TOTAL HEIGHT	68"
TOTAL WEIGHT	47.62 lbs

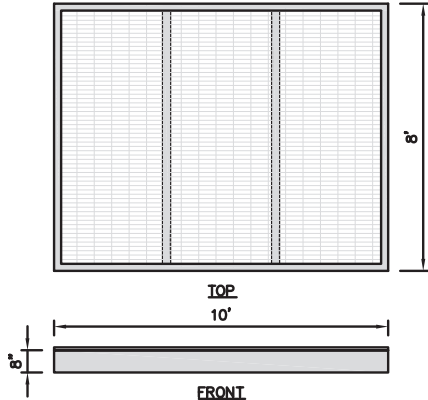
- NOTES:
- (EQL.2 SHOWN FOR CLARITY PURPOSES)
 - ALL DIMENSIONS SHOW AS "TYP" WILL BE THE SAME FOR ALL LADDER ASSEMBLIES



STEEL PLATFORM LADDER DETAIL

4

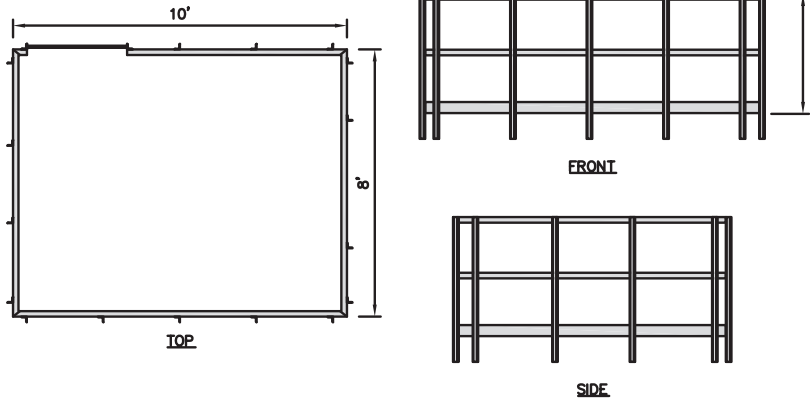
COMMSCOPE - EQP810 STEEL PLATFORM	
TOTAL WEIGHT	±1017.8 lbs
MAX LOAD CAPACITY	10,000 lbs



STEEL PLATFORM DETAIL

5

COMMSCOPE EQHR810 PLATFORM HANDRAIL KIT	
TOTAL WEIGHT	±645.09 lbs



PLATFORM HANDRAIL KIT DETAIL

6

dish
wireless.

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LITTLETON, CO 80120

FULLERTON

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SUBMITTALS		
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A	03/31/2022	ISSUED FOR REVIEW
B	05/11/2022	ISSUED FOR REVIEW
O	07/07/2022	ISSUED FOR PERMITTING

A&E PROJECT NUMBER
2021.0030.0004

DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
EQUIPMENT PLATFORM AND
H-FRAME DETAILS

SHEET NUMBER

A-5

NOT USED

NO SCALE

2

NOT USED

NO SCALE

3

CHARLES INDUSTRY HEX CUBE-PM639155N4 <table><tr><td>DIMENSIONS (HxWxD):</td><td>74"x32"x32"</td></tr><tr><td>POWER PLANT:</td><td>-48VDC ABB/600W</td></tr><tr><td>TOTAL WEIGHT (EMPTY)</td><td>408 LBS</td></tr></table> PLAN SIDE BACK SIDE FRONT	DIMENSIONS (HxWxD):	74"x32"x32"	POWER PLANT:	-48VDC ABB/600W	TOTAL WEIGHT (EMPTY)	408 LBS	SQUARE D SAFETY SWITCHES D224NRB <table><tr><td>ENCLOSURE DIM (HxWxD)</td><td>29.25"x19.00"x8.50"</td></tr><tr><td>ENCLOSURE TYPE</td><td>NEMA 3R RAINPROOF</td></tr><tr><td>UL LISTED</td><td>FILE E-2875</td></tr></table> SIDE TOP FRONT	ENCLOSURE DIM (HxWxD)	29.25"x19.00"x8.50"	ENCLOSURE TYPE	NEMA 3R RAINPROOF	UL LISTED	FILE E-2875	RAYCAP PPC RDIAC-2465-P-240-MTS <table><tr><td>ENCLOSURE DIMENSIONS (HxWxD):</td><td>39"x22.855"x12.593</td></tr><tr><td>WEIGHT:</td><td>80 lbs</td></tr><tr><td>OPERATING AC VOLTAGE</td><td>240/120 1 PHASE 3W+G</td></tr></table> TOP BACK SIDE FRONT SIDE	ENCLOSURE DIMENSIONS (HxWxD):	39"x22.855"x12.593	WEIGHT:	80 lbs	OPERATING AC VOLTAGE	240/120 1 PHASE 3W+G
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CABINET DETAIL NO SCALE 1	SAFETY SWITCH DETAIL NO SCALE 2	POWER PROTECTION CABINET (PPC) DETAIL NO SCALE 3																		
LEVITON 1N240-21 SUBMETER <table><tr><td>WATTAGE</td><td>3</td></tr><tr><td>AMPS</td><td>200</td></tr><tr><td>VOLTS AC</td><td>120, 208, 240</td></tr><tr><td>PHASE</td><td>1</td></tr></table> PLAN BACK SIDE FRONT	WATTAGE	3	AMPS	200	VOLTS AC	120, 208, 240	PHASE	1	PCTEL GPSGL-TMG-SPI-40NCB <table><tr><td>DIMENSIONS (DIAxH) MM/INCH</td><td>81x184mm 3.2"x7.25"</td></tr><tr><td>WEIGHT W/ACCESSORIES</td><td>075 lbs</td></tr><tr><td>CONNECTOR</td><td>N-FEMALE</td></tr><tr><td>FREQUENCY RANGE</td><td>1590 ± 30MHz</td></tr></table> TOP BACK SIDE	DIMENSIONS (DIAxH) MM/INCH	81x184mm 3.2"x7.25"	WEIGHT W/ACCESSORIES	075 lbs	CONNECTOR	N-FEMALE	FREQUENCY RANGE	1590 ± 30MHz	MINIMUM OF 75% OR 270° IN ANY DIRECTION GPS GPS UNIT OBSTRUCTIONS MUST BE BELOW 10°		
WATTAGE	3																			
AMPS	200																			
VOLTS AC	120, 208, 240																			
PHASE	1																			
DIMENSIONS (DIAxH) MM/INCH	81x184mm 3.2"x7.25"																			
WEIGHT W/ACCESSORIES	075 lbs																			
CONNECTOR	N-FEMALE																			
FREQUENCY RANGE	1590 ± 30MHz																			
E-MON D-MON DETAIL NO SCALE 4	GPS DETAIL NO SCALE 5	GPS MINIMUM SKY VIEW REQUIREMENTS NO SCALE 6																		
CU12PSM6P4XXX (4 AWG CONDUCTORS) CU12PSM9P6XXX (6 AWG CONDUCTORS) CU12PSM9P8XXX (8 AWG CONDUCTORS)	CHARLES CFIT-PF2020DSH1 FIBER TELCO ENCLOSURE <table><tr><td>ENCLOSURE DIMS (HxWxD)</td><td>20"x20"x9"</td></tr><tr><td>ENCLOSURE WEIGHT</td><td>20 lbs</td></tr><tr><td>MOUNTING</td><td>WALL</td></tr><tr><td>COMPLIANCE</td><td>TYPE 4</td></tr></table> FRONT SIDE BACK FRONT	ENCLOSURE DIMS (HxWxD)	20"x20"x9"	ENCLOSURE WEIGHT	20 lbs	MOUNTING	WALL	COMPLIANCE	TYPE 4											
ENCLOSURE DIMS (HxWxD)	20"x20"x9"																			
ENCLOSURE WEIGHT	20 lbs																			
MOUNTING	WALL																			
COMPLIANCE	TYPE 4																			
CABLES UNLIMITED HYBRID CABLE MINIMUM BEND RADIUS NO SCALE 7	FIBER TELCO ENCLOSURE DETAIL NO SCALE 8	NOT USED NO SCALE 9																		

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WISCONSIN

DANIEL W. SMITH

44096-6

SCHAUMBURG, IL

PROFESSIONAL ENGINEER

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CHECKED BY:

APPROVED BY:

LA

DZ

DS

RFDS REV #: 2

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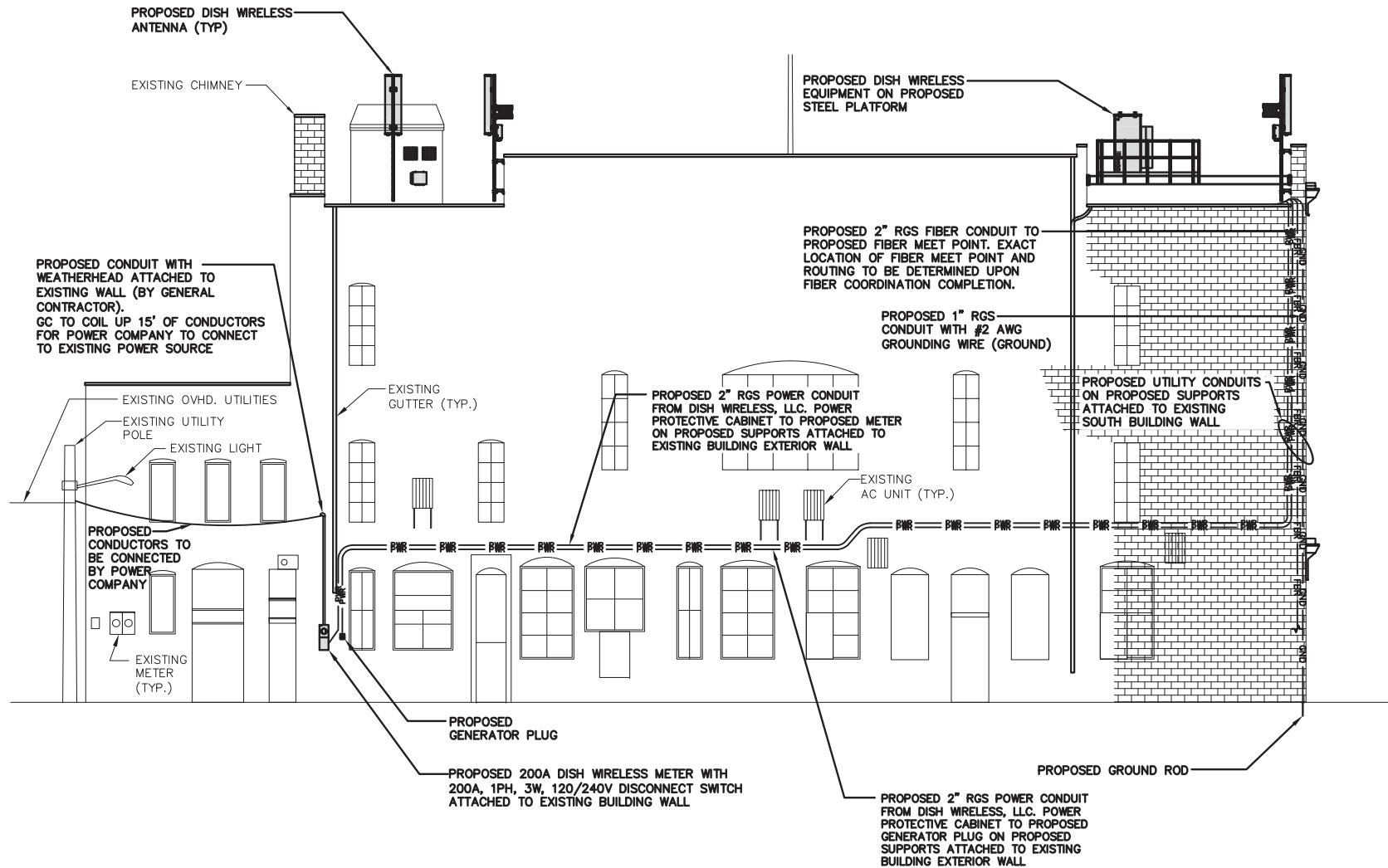
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DISH Wireless L.L.C.
PROJECT INFORMATION

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314 6TH STREET
RACINE, WI 53403

SHEET TITLE
EQUIPMENT DETAILS

SHEET NUMBER
A-6

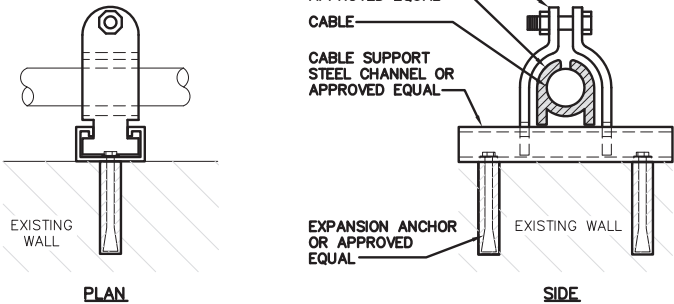


UTILITY RISER DIAGRAM

NO SCALE 1

CABLE/CONDUIT WALL MOUNT SUPPORT	
UNISTRUT TYPE	GRAINGER P1000T
UNISTRUT DIMENSIONS (HxWxD)	1 5/8" x 1 5/8" x 12"
WEIGHT/ VOLUME	1.85 LB PER FOOT
ULTIMATE LOAD CAPACITY	718.5 LB

NOTES:
1. ALL HARDWARE AND PARTS SHALL BE HOT DIPPED GALVANIZED WITH LOCK WASHERS AND HEAVY HEX NUTS.
2. VERTICAL CABLE MOUNT: 4'-0" MAX. UNISTRUT SEPARATION
3. HORIZONTAL CABLE MOUNT: 3'-0" MAX UNISTRUT SEPARATION



CABLE/CONDUIT WALL MOUNT SUPPORT DETAIL NO SCALE 2

NOT USED NO SCALE 3



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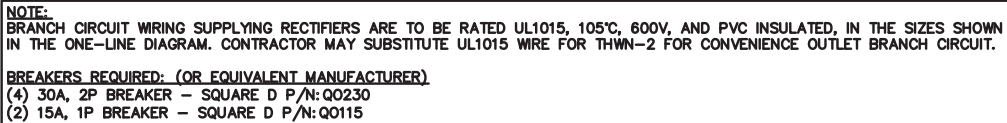


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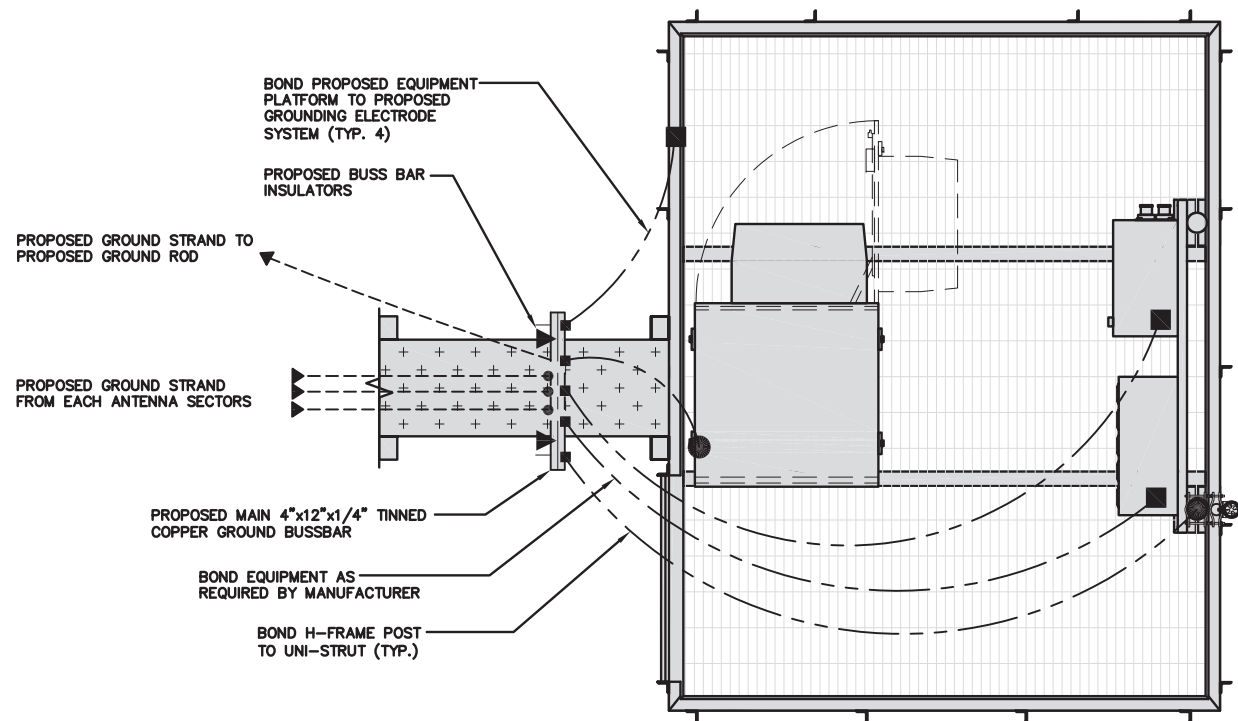
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MWMKE00116B 314 6TH STREET RACINE, WI 53403		
SHEET TITLE		
UTILITY RISER DIAGRAM		
SHEET NUMBER		
E-1		



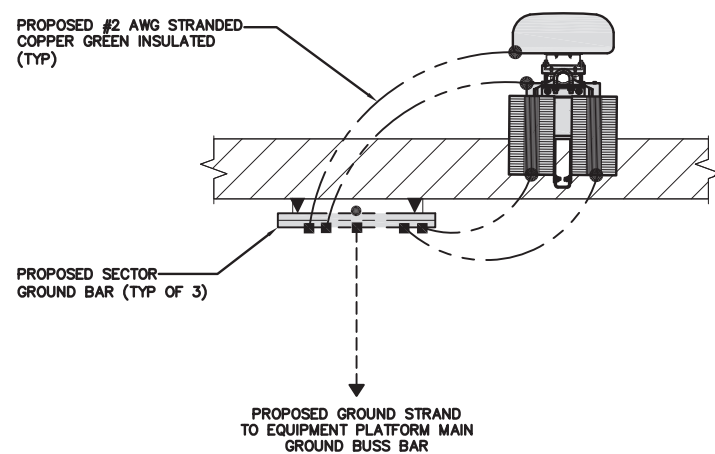
E-3



TYPICAL ROOFTOP EQUIPMENT GROUNDING PLAN

NO SCALE

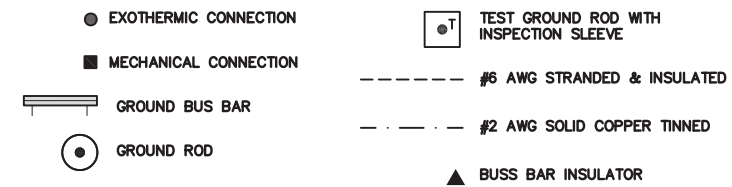
1



GROUNDING PLAN

NO SCALE

2



GROUNDING LEGEND

- GROUNDING IS SHOWN DIAGRAMMATICALLY ONLY.
- CONTRACTOR SHALL GROUND ALL EQUIPMENT AS A COMPLETE SYSTEM. GROUNDING SHALL BE IN COMPLIANCE WITH NEC SECTION 250 AND DISH Wireless L.L.C. GROUNDING AND BONDING REQUIREMENTS AND MANUFACTURER'S SPECIFICATIONS.
- ALL GROUND CONDUCTORS SHALL BE COPPER; NO ALUMINUM CONDUCTORS SHALL BE USED.
- NO EXOTHERMIC WELDING ON ROOFTOP

GROUNDING ROOFTOP KEY NOTES

- (A) **EXTERIOR GROUND RING:** #2 AWG SOLID COPPER, BURIED AT A DEPTH OF AT LEAST 30 INCHES BELOW GRADE, OR 6 INCHES BELOW THE FROST LINE AND APPROXIMATELY 24 INCHES FROM THE EXTERIOR WALL OR FOOTING.
- (B) **ROOFTOP GROUND SYSTEM:** THE GROUND SYSTEM USING MINIMUM #2 AWG SOLID COPPER CONDUCTORS.
- (C) **INTERIOR GROUND RING:** #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTOR EXTENDED AROUND THE PERIMETER OF THE EQUIPMENT AREA. ALL NON-TELECOMMUNICATIONS RELATED METALLIC OBJECTS FOUND WITHIN A SITE SHALL BE GROUNDED TO THE INTERIOR GROUND RING WITH #6 AWG STRANDED GREEN INSULATED CONDUCTOR.
- (D) **BOND TO INTERIOR GROUND RING:** #2 AWG SOLID TINNED COPPER WIRE PRIMARY BONDS SHALL BE PROVIDED AT LEAST AT FOUR POINTS ON THE INTERIOR GROUND RING, LOCATED AT THE CORNERS OF THE BUILDING OR ROOM.
- (E) **GROUND ROD:** UL LISTED COPPER CLAD STEEL. MINIMUM 1/2" DIAMETER BY EIGHT FEET LONG. GROUND RODS SHALL BE INSTALLED WITH INSPECTION SLEEVES. GROUND RODS SHALL BE DRIVEN TO THE DEPTH OF GROUND RING CONDUCTOR.
- (F) **CELL REFERENCE GROUND BAR (CRGB):** POINT OF GROUND REFERENCE FOR ALL COMMUNICATIONS EQUIPMENT FRAMES. ALL BONDS ARE MADE WITH #2 AWG UNLESS NOTED OTHERWISE STRANDED GREEN INSULATED COPPER CONDUCTORS. BOND TO COMMON BUILDING GROUND SYSTEM WITH (2) #2 SOLID TINNED COPPER CONDUCTORS.
- (G) **HATCH PLATE GROUND BAR:** BOND TO THE COMMON BUILDING GROUND SYSTEM WITH TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS. WHEN A HATCH-PLATE AND A CELL REFERENCE GROUND BAR ARE BOTH PRESENT, THE CRGB MUST BE CONNECTED TO THE HATCH-PLATE AND TO THE INTERIOR GROUND RING USING (2) TWO #2 AWG STRANDED GREEN INSULATED COPPER CONDUCTORS EACH.
- (H) **EXTERIOR CABLE ENTRY PORT GROUND BARS:** LOCATED AT THE ENTRANCE TO THE CELL SITE ROOM. BOND TO GROUND RING WITH A #2 AWG SOLID TINNED COPPER CONDUCTORS WITH MECHANICAL CONNECTIONS.
- (I) **TELCO GROUND BAR:** BOND TO BOTH CELL REFERENCE GROUND BAR OR EXTERIOR GROUND RING.
- (J) **FRAME BONDING:** THE BONDING POINT FOR TELECOM EQUIPMENT FRAMES SHALL BE THE GROUND BUS THAT IS NOT ISOLATED FROM THE EQUIPMENTS METAL FRAMEWORK.
- (K) **INTERIOR UNIT BONDS:** METAL FRAMES, CABINETS AND INDIVIDUAL METALLIC UNITS LOCATED WITH THE AREA OF THE INTERIOR GROUND RING REQUIRE A #6 AWG STRANDED GREEN INSULATED COPPER BOND TO THE INTERIOR GROUND RING.
- (L) **FENCE AND GATE GROUNDING:** METAL FENCES SHALL BE BONDED TO THE COMMON BUILDING GROUND SYSTEM WITH A #2 AWG SOLID TINNED COPPER CONDUCTOR AT AN INTERVAL NOT EXCEEDING 25 FEET. BONDS SHALL BE MADE AT EACH GATE POST AND ACROSS GATE OPENINGS.
- (M) **EXTERIOR UNIT BONDS:** METALLIC OBJECTS, EXTERNAL TO OR MOUNTED TO THE BUILDING, SHALL BE BONDED TO THE COMMON BUILDING GROUND SYSTEM. USING #2 TINNED SOLID COPPER WIRE
- (N) **ICE BRIDGE SUPPORTS:** EACH ICE BRIDGE LEG SHALL BE BONDED TO THE GROUND RING WITH #2 AWG BARE TINNED COPPER CONDUCTOR. PROVIDE EXOTHERMIC WELDS AT BOTH THE ICE BRIDGE LEG AND BURIED GROUND RING.
- (O) **DURING ALL DC POWER SYSTEM CHANGES** INCLUDING DC SYSTEM CHANGE OUTS, RECTIFIER REPLACEMENTS OR ADDITIONS, BREAKER DISTRIBUTION CHANGES, BATTERY ADDITIONS, BATTERY REPLACEMENTS AND INSTALLATIONS OR CHANGES TO DC CONVERTER SYSTEMS IT SHALL BE REQUIRED THAT SERVICE CONTRACTORS VERIFY ALL DC POWER SYSTEMS ARE EQUIPPED WITH A MASTER DC SYSTEM RETURN GROUND CONDUCTOR FROM THE DC POWER SYSTEM COMMON RETURN BUS DIRECTLY CONNECTED TO THE CELL SITE REFERENCE GROUND BAR
- (P) **ROOFTOP COLLECTOR BUSS BAR** IS TO BE MECHANICALLY BONDED TO COMMON BUILDING GROUND SYSTEM.
- REFER TO DISH Wireless L.L.C. GROUNDING NOTES.

GROUNDING KEY NOTES

NO SCALE

3

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RACINE, WI 53403

SHEET TITLE
GROUNDING PLANS
AND NOTES

SHEET NUMBER

G-1

1. EXOTHERMIC WELD (2) TWO, #2 AWG BARE TINNED SOLID COPPER CONDUCTORS TO GROUND BAR. ROUTE CONDUCTORS TO BURIED GROUND RING AND PROVIDE PARALLEL EXOTHERMIC WELD.

2. ALL EXTERIOR GROUNDING HARDWARE SHALL BE STAINLESS STEEL 3/8" DIAMETER OR LARGER. ALL HARDWARE 18-8 STAINLESS STEEL INCLUDING LOCK WASHERS, COAT ALL SURFACES WITH AN ANTI-OXIDANT COMPOUND BEFORE MATING.

3. FOR GROUND BOND TO STEEL ONLY: COAT ALL SURFACES WITH AN ANTI-OXIDANT COMPOUND BEFORE MATING.

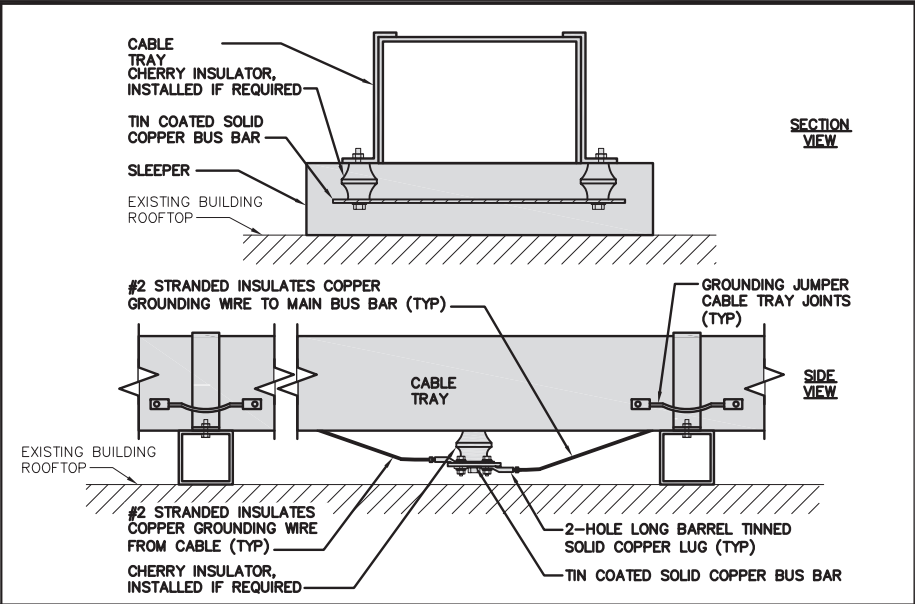
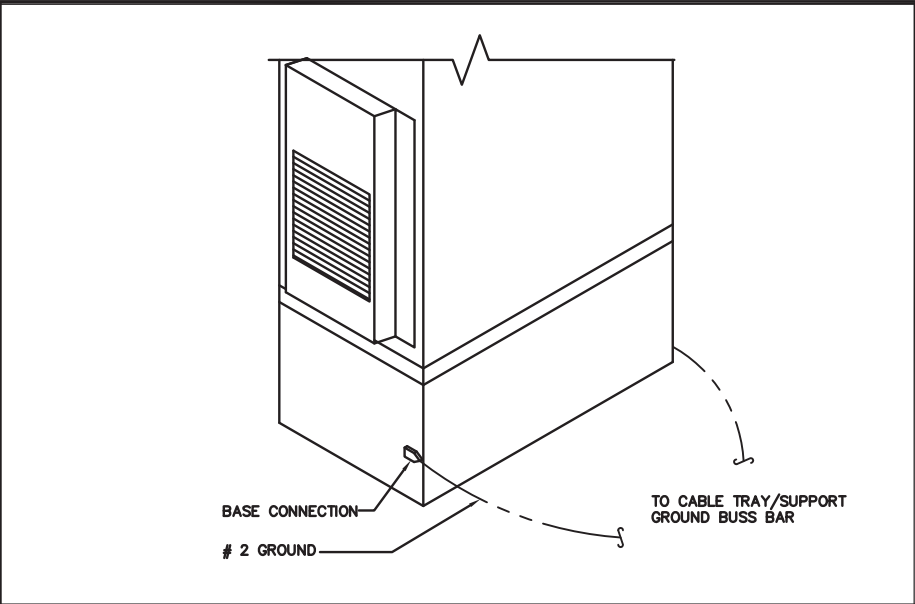
4. DO NOT INSTALL CABLE GROUNDING KIT AT A BEND AND ALWAYS DIRECT GROUND CONDUCTOR DOWN TO GROUNDING BUS.

5. NUT & WASHER SHALL BE PLACED ON THE FRONT SIDE OF THE GROUND BAR AND BOLTED ON THE BACK SIDE.

6. ALL GROUNDING PARTS AND EQUIPMENT TO BE SUPPLIED AND INSTALLED BY CONTRACTOR.

7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLING ADDITIONAL GROUND BAR AS REQUIRED.

8. ENSURE THE WIRE INSULATION TERMINATION IS WITHIN 1/8" OF THE BARREL (NO SHINERS).



TYPICAL GROUNDING NOTES

NO SCALE

1

OUTDOOR CABINET GROUNDING

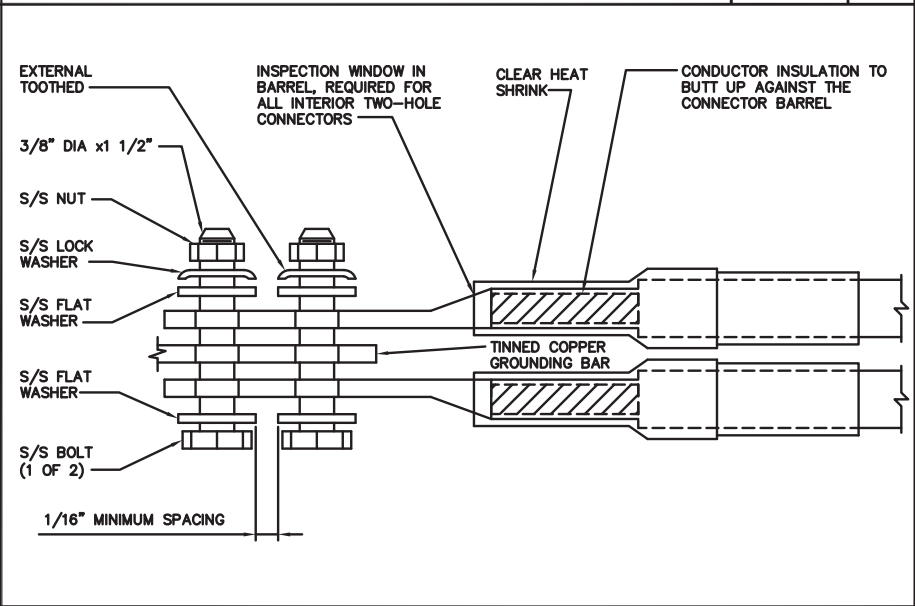
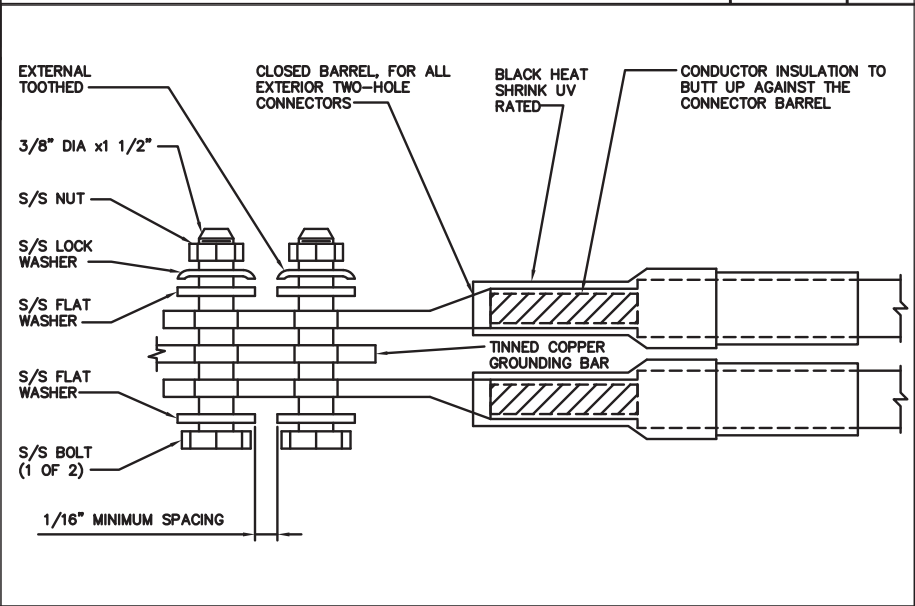
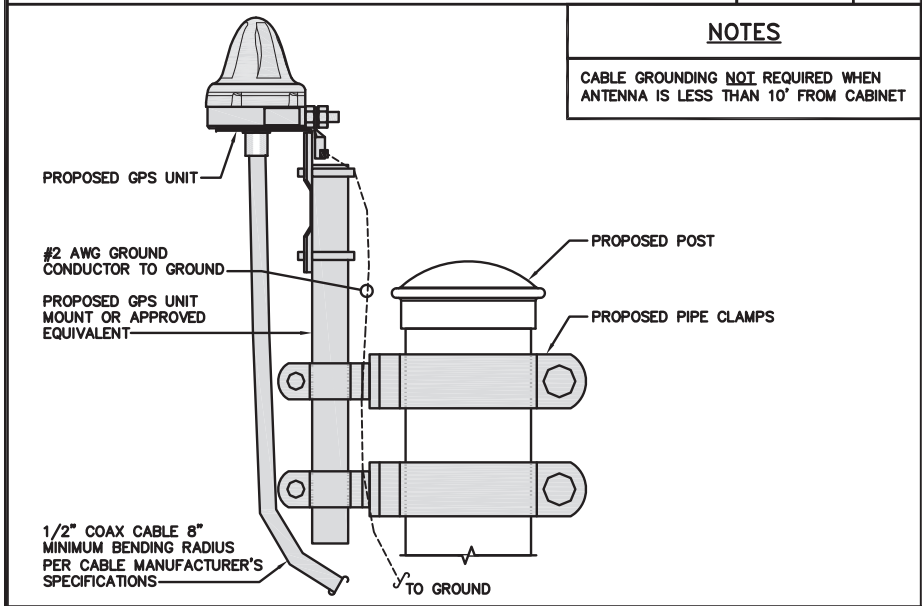
NO SCALE

2

TYPICAL CABLE TRAY GROUND BUSS BAR

NO SCALE

3



TYPICAL GPS UNIT GROUNDING

NO SCALE

4

TYPICAL EXTERIOR TWO HOLE LUG

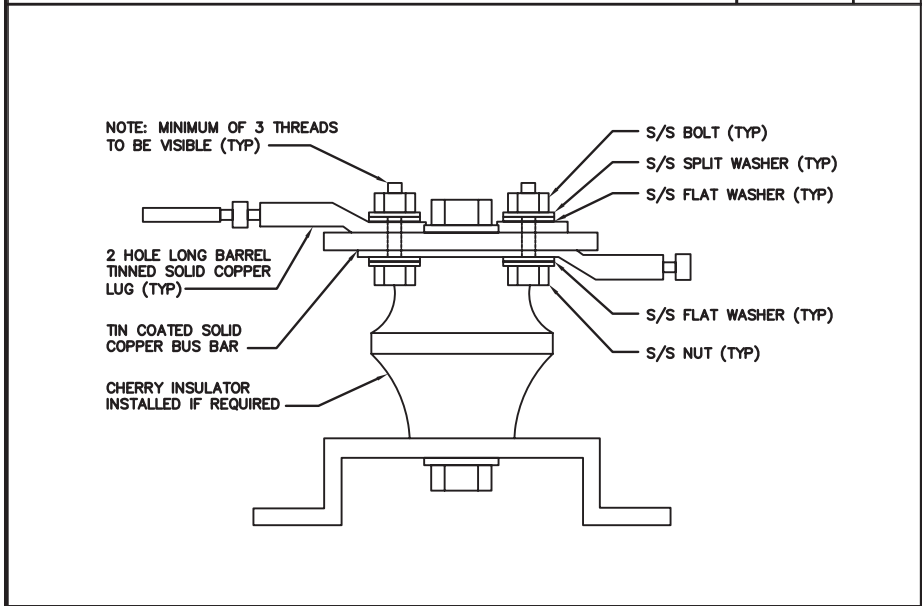
NO SCALE

5

TYPICAL INTERIOR TWO HOLE LUG

NO SCALE

6



NOT USED

NOT USED

LUG DETAIL

NO SCALE

7

NOT USED

NO SCALE

8

NOT USED

NO SCALE

9



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SHEET TITLE
GROUNDING DETAILS

SHEET NUMBER
G-2

HYBRID/DISCREET CABLES		3/4" TAPE WIDTHS WITH 3/4" SPACING											
LOW-BAND RRH (600 MHz N71 BASEBAND) + (850 MHz N26 BAND) + (700 MHz N29 BAND) – OPTIONAL PER MARKET ADD FREQUENCY COLOR TO SECTOR BAND (CBRS WILL USE YELLOW BAND)	ALPHA RRH				BETA RRH				GAMMA RRH				
	PORT 1 + SLANT	PORT 2 – SLANT	PORT 3 + SLANT	PORT 4 – SLANT	PORT 1 + SLANT	PORT 2 – SLANT	PORT 3 + SLANT	PORT 4 – SLANT	PORT 1 + SLANT	PORT 2 – SLANT	PORT 3 + SLANT	PORT 4 – SLANT	
	RED	RED	RED	RED	BLUE	BLUE	BLUE	BLUE	GREEN	GREEN	GREEN	GREEN	
	ORANGE	ORANGE	RED	RED	ORANGE	ORANGE	BLUE	BLUE	ORANGE	ORANGE	GREEN	GREEN	
MID-BAND RRH (AWS BANDS N66+N70) ADD FREQUENCY COLOR TO SECTOR BAND (CBRS WILL USE YELLOW BANDS)													
		WHITE (–) PORT	ORANGE	ORANGE		WHITE (–) PORT	ORANGE	ORANGE		WHITE (–) PORT	ORANGE	ORANGE	
				WHITE (–) PORT				WHITE (–) PORT				WHITE (–) PORT	
HYBRID/DISCREET CABLES		EXAMPLE 1	EXAMPLE 2	EXAMPLE 3	CANISTER COAX #1	CANISTER COAX #2							
INCLUDE SECTOR BANDS BEING SUPPORTED ALONG WITH FREQUENCY BANDS.													
EXAMPLE 1 – HYBRID, OR DISCREET, SUPPORTS ALL SECTORS, BOTH LOW-BANDS AND MID-BANDS.													
EXAMPLE 2 – HYBRID, OR DISCREET, SUPPORTS CBRS ONLY, ALL SECTORS.													
EXAMPLE 3 – MAIN COAX WITH GROUND MOUNTED RRHs.													
FIBER JUMPERS TO RRHs		LOW BAND RRH	MID BAND RRH	LOW BAND RRH	MID BAND RRH	LOW BAND RRH	MID BAND RRH						
LOW-BAND HHR FIBER CABLES HAVE SECTOR STRIPE ONLY.													
POWER CABLES TO RRHs		LOW BAND RRH	MID BAND RRH	LOW BAND RRH	MID BAND RRH	LOW BAND RRH	MID BAND RRH						
LOW-BAND RRH POWER CABLES HAVE SECTOR STRIPE ONLY													
RET MOTORS AT ANTENNAS		ANTENNA 1 MID BAND	ANTENNA 1 LOW BAND	ANTENNA 1 MID BAND	ANTENNA 1 LOW BAND	ANTENNA 1 MID BAND	ANTENNA 1 LOW BAND						
RET CONTROL IS HANDLED BY THE MID-BAND RRH WHEN ONE SET OF RET PORTS EXIST ON ANTENNA.													
SEPARATE RET CABLES ARE USED WHEN ANTENNA PORTS PROVIDE INPUTS FOR BOTH LOW AND MID BANDS.													
MICROWAVE RADIO LINKS		FORWARD AZIMUTH OF 0–120 DEGREES PRIMARY SECONDARY		FORWARD AZIMUTH OF 120–240 DEGREES PRIMARY SECONDARY		FORWARD AZIMUTH OF 240–359 DEGREES PRIMARY SECONDARY							
LINKS WILL HAVE A 1.5–2 INCH WHITE WRAP WITH THE AZIMUTH COLOR OVERLAPPING IN THE MIDDLE. ADD ADDITIONAL SECTOR COLOR BANDS FOR EACH ADDITIONAL MW RADIO.													
MICROWAVE CABLES WILL REQUIRE P-TOUCH LABELS INSIDE THE CABINET TO IDENTIFY THE LOCAL AND REMOTE SITE ID's.													

RF CABLE COLOR CODES

NO SCALE

1

NOT USED

NO SCALE

4

LOW BANDS (N71+N26)
OPTIONAL – (N29)

ORANGE

CBRS TECH
(3 GHz)

YELLOW

AWS
(N66+N70+H-BLOCK)

PURPLE

NEGATIVE SLANT PORT
ON ANT/RRH

WHITE

ALPHA SECTOR

RED

BETA SECTOR

BLUE

GAMMA SECTOR

GREEN

COLOR IDENTIFIER

NO SCALE

2

NOT USED

NO SCALE

3

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SHEET TITLE
RF
CABLE COLOR CODE

SHEET NUMBER

RF-1

EXOTHERMIC CONNECTION
MECHANICAL CONNECTION
BUSS BAR INSULATOR
CHEMICAL ELECTROLYTIC GROUNDING SYSTEM
TEST CHEMICAL ELECTROLYTIC GROUNDING SYSTEM
EXOTHERMIC WITH INSPECTION SLEEVE
GROUNDING BAR
GROUND ROD
TEST GROUND ROD WITH INSPECTION SLEEVE

SINGLE POLE SWITCH

DUPLEX RECEPTACLE

DUPLEX GFCI RECEPTACLE

FLUORESCENT LIGHTING FIXTURE (2) TWO LAMPS 48-T8

SMOKE DETECTION (DC)

EMERGENCY LIGHTING (DC)

SECURITY LIGHT W/PHOTOCELL LITHONIA ALXW
LED-1-25A400/51K-SR4-120-PE-DDBTXD

CHAIN LINK FENCE

WOOD/WROUGHT IRON FENCE

WALL STRUCTURE

LEASE AREA

PROPERTY LINE (PL)

SETBACKS

ICE BRIDGE

CABLE TRAY

WATER LINE

UNDERGROUND POWER

UNDERGROUND TELCO

OVERHEAD POWER

OVERHEAD TELCO

UNDERGROUND TELCO/POWER

ABOVE GROUND POWER

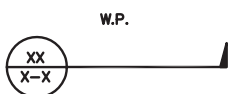
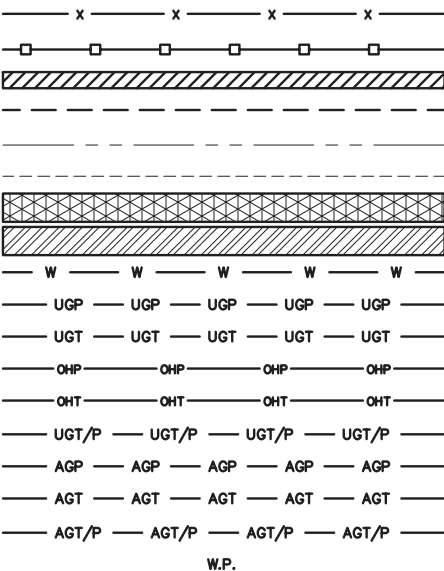
ABOVE GROUND TELCO

ABOVE GROUND TELCO/POWER

WORKPOINT

SECTION REFERENCE

DETAIL REFERENCE



AB ANCHOR BOLT
ABV ABOVE
AC ALTERNATING CURRENT
ADDL ADDITIONAL
AFF ABOVE FINISHED FLOOR
AFG ABOVE FINISHED GRADE
AGL ABOVE GROUND LEVEL
AIC AMPERAGE INTERRUPTION CAPACITY
ALUM ALUMINUM
ALT ALTERNATE
ANT ANTENNA
APPROX APPROXIMATE
ARCH ARCHITECTURAL
ATS AUTOMATIC TRANSFER SWITCH
AWG AMERICAN WIRE GAUGE
BATT BATTERY
BLDG BUILDING
BLK BLOCK
BLKG BLOCKING
BM BEAM
BTC BARE TINNED COPPER CONDUCTOR
BOF BOTTOM OF FOOTING
CAB CABINET
CANT CANTILEVERED
CHG CHARGING
CLG CEILING
CLR CLEAR
COL COLUMN
COMM COMMON
CONC CONCRETE
CONSTR CONSTRUCTION
DBL DOUBLE
DC DIRECT CURRENT
DEPT DEPARTMENT
DF DOUGLAS FIR
DIA DIAMETER
DIAG DIAGONAL
DIM DIMENSION
DWG DRAWING
DWL DOWEL
EA EACH
EC ELECTRICAL CONDUCTOR
EL ELEVATION
ELEC ELECTRICAL
EMT ELECTRICAL METALLIC TUBING
ENG ENGINEER
EQ EQUAL
EXP EXPANSION
EXT EXTERIOR
EW EACH WAY
FAB FABRICATION
FF FINISH FLOOR
FG FINISH GRADE
FIF FACILITY INTERFACE FRAME
FIN FINISH(ED)
FLR FLOOR
FDN FOUNDATION
FOC FACE OF CONCRETE
FOM FACE OF MASONRY
FOS FACE OF STUD
FOW FACE OF WALL
FS FINISH SURFACE
FT FOOT
FTG FOOTING
GA GAUGE
GEN GENERATOR
GFCI GROUND FAULT CIRCUIT INTERRUPTER
GLB GLUE LAMINATED BEAM
GLV GALVANIZED
GPS GLOBAL POSITIONING SYSTEM
GND GROUND
GSM GLOBAL SYSTEM FOR MOBILE
HDG HOT DIPPED GALVANIZED
HDR HEADER
HGR HANGER
HVAC HEAT/VENTILATION/AIR CONDITIONING
HT HEIGHT
IGR INTERIOR GROUND RING

IN INCH
INT INTERIOR
LB(S) POUND(S)
LF LINEAR FEET
LTE LONG TERM EVOLUTION
MAS MASONRY
MAX MAXIMUM
MB MACHINE BOLT
MECH MECHANICAL
MFR MANUFACTURER
MGB MASTER GROUND BAR
MIN MINIMUM
MISC MISCELLANEOUS
MTL METAL
MTS MANUAL TRANSFER SWITCH
MW MICROWAVE
NEC NATIONAL ELECTRIC CODE
NM NEWTON METERS
NO. NUMBER
NUMBER
NTS NOT TO SCALE
OC ON-CENTER
OSHA OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION
OPNG OPENING
P/C PRECAST CONCRETE
PCS PERSONAL COMMUNICATION SERVICES
PCU PRIMARY CONTROL UNIT
PRC PRIMARY RADIO CABINET
PP POLARIZING PRESERVING
PSF POUNDS PER SQUARE FOOT
PSI POUNDS PER SQUARE INCH
PT PRESSURE TREATED
PWR POWER CABINET
QTY QUANTITY
RAD RADIUS
RECT RECTIFIER
REF REFERENCE
REINF REINFORCEMENT
REQ'D REQUIRED
RET REMOTE ELECTRIC TILT
RF RADIO FREQUENCY
RMC RIGID METALLIC CONDUIT
RRH REMOTE RADIO HEAD
RRU REMOTE RADIO UNIT
RWY RACEWAY
SCH SCHEDULE
SHT SHEET
SIAD SMART INTEGRATED ACCESS DEVICE
SIM SIMILAR
SPEC SPECIFICATION
SQ SQUARE
SS STAINLESS STEEL
STD STANDARD
STL STEEL
TEMP TEMPORARY
THK THICKNESS
TMA TOWER MOUNTED AMPLIFIER
TN TOE NAIL
TOA TOP OF ANTENNA
TOC TOP OF CURB
TOF TOP OF FOUNDATION
TOP TOP OF PLATE (PARAPET)
TOS TOP OF STEEL
TOW TOP OF WALL
TVSS TRANSIENT VOLTAGE SURGE SUPPRESSION
TYP TYPICAL
UG UNDERGROUND
UL UNDERWRITERS LABORATORY
UNO UNLESS NOTED OTHERWISE
UMTS UNIVERSAL MOBILE TELECOMMUNICATIONS SYSTEM
UPS UNINTERRUPTIBLE POWER SYSTEM (DC POWER PLANT)
VIF VERIFIED IN FIELD
W WIDE
W/ WITH
WD WOOD
WP WEATHERPROOF
WT WEIGHT



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DRAWN BY: CHECKED BY: APPROVED BY:

LA DZ DS

RFDS REV #: 2

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A&E PROJECT NUMBER
2021.0030.0004

DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
LEGEND AND
ABBREVIATIONS

SHEET NUMBER

GN-1

LEGEND

ABBREVIATIONS

SIGN TYPES		
TYPE	COLOR	COLOR CODE PURPOSE
INFORMATION	GREEN	"INFORMATIONAL SIGN" TO NOTIFY OTHERS OF SITE OWNERSHIP & CONTACT NUMBER AND POTENTIAL RF EXPOSURE.
NOTICE	BLUE	"NOTICE BEYOND THIS POINT" RF FIELDS BEYOND THIS POINT MAY EXCEED THE FCC GENERAL PUBLIC EXPOSURE LIMIT. OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)
CAUTION	YELLOW	"CAUTION BEYOND THIS POINT" RF FIELDS BEYOND THIS POINT MAY EXCEED THE FCC GENERAL PUBLIC EXPOSURE LIMIT. OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)
WARNING	ORANGE/RED	"WARNING BEYOND THIS POINT" RF FIELDS AT THIS SITE EXCEED FCC RULES FOR HUMAN EXPOSURE. FAILURE TO OBEY ALL POSTED SIGNS AND SITE GUIDELINES FOR WORKING IN RF ENVIRONMENTS COULD RESULT IN SERIOUS INJURY. IN ACCORDANCE WITH FEDERAL COMMUNICATIONS COMMISSION RULES ON RADIO FREQUENCY EMISSIONS 47 CFR-1.1307(b)

SIGN PLACEMENT:

- RF SIGNAGE PLACEMENT SHALL FOLLOW THE RECOMMENDATIONS OF AN EXISTING EME REPORT, CREATED BY A THIRD PARTY PREVIOUSLY AUTHORIZED BY DISH Wireless L.L.C.
- INFORMATION SIGN (GREEN) SHALL BE LOCATED ON EXISTING DISH Wireless L.L.C. EQUIPMENT.
 - A) IF THE INFORMATION SIGN IS A STICKER, IT SHALL BE PLACED ON EXISTING DISH Wireless L.L.C. EQUIPMENT CABINET.
 - B) IF THE INFORMATION SIGN IS A METAL SIGN IT SHALL BE PLACED ON EXISTING DISH Wireless L.L.C. H-FRAME WITH A SECURE ATTACH METHOD.
- IF EME REPORT IS NOT AVAILABLE AT THE TIME OF CREATION OF CONSTRUCTION DOCUMENTS; PLEASE CONTACT DISH Wireless L.L.C. CONSTRUCTION MANAGER FOR FURTHER INSTRUCTION ON HOW TO PROCEED.

NOTES:

1. FOR DISH Wireless L.L.C. LOGO, SEE DISH Wireless L.L.C. DESIGN SPECIFICATIONS (PROVIDED BY DISH Wireless L.L.C.)
2. SITE ID SHALL BE APPLIED TO SIGNS USING "LASER ENGRAVING" OR ANY OTHER WEATHER RESISTANT METHOD (DISH Wireless L.L.C. APPROVAL REQUIRED)
3. TEXT FOR SIGNAGE SHALL INDICATE CORRECT SITE NAME AND NUMBER AS PER DISH Wireless L.L.C. CONSTRUCTION MANAGER RECOMMENDATIONS.
4. CABINET/SHELTER MOUNTING APPLICATION REQUIRES ANOTHER PLATE APPLIED TO THE FACE OF THE CABINET WITH WATER PROOF POLYURETHANE ADHESIVE
5. ALL SIGNS WILL BE SECURED WITH EITHER STAINLESS STEEL ZIP TIES OR STAINLESS STEEL TECH SCREWS
6. ALL SIGNS TO BE 8.5"x11" AND MADE WITH 0.04" OF ALUMINUM MATERIAL

INFORMATION

This is an access point to an area with transmitting antennas.

Obey all signs and barriers beyond this point.
Call the DISH Wireless L.L.C. NOC at 1-866-624-6874

Site ID: _____



THIS SIGN IS FOR REFERENCE PURPOSES ONLY

NOTICE



Transmitting Antenna(s)

Radio frequency fields beyond this point **MAY EXCEED** the FCC Occupational exposure limit.

Obey all posted signs and site guidelines for working in radio frequency environments.

Call the DISH Wireless L.L.C. NOC at 1-866-624-6874 prior to working beyond this point.

Site ID: _____



THIS SIGN IS FOR REFERENCE PURPOSES ONLY

CAUTION



Transmitting Antenna(s)

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WARNING



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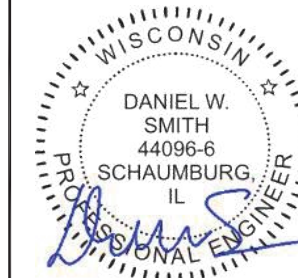
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DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
RF SIGNAGE

SHEET NUMBER
GN-2

SITE ACTIVITY REQUIREMENTS:

1. NOTICE TO PROCEED – NO WORK SHALL COMMENCE PRIOR TO CONTRACTOR RECEIVING A WRITTEN NOTICE TO PROCEED (NTP) AND THE ISSUANCE OF A PURCHASE ORDER. PRIOR TO ACCESSING/ENTERING THE SITE YOU MUST CONTACT THE DISH Wireless L.L.C. AND TOWER OWNER NOC & THE DISH Wireless L.L.C. AND TOWER OWNER CONSTRUCTION MANAGER.
2. "LOOK UP" – DISH Wireless L.L.C. AND TOWER OWNER SAFETY CLIMB REQUIREMENT:

THE INTEGRITY OF THE SAFETY CLIMB AND ALL COMPONENTS OF THE CLIMBING FACILITY SHALL BE CONSIDERED DURING ALL STAGES OF DESIGN, INSTALLATION, AND INSPECTION. TOWER MODIFICATION, MOUNT REINFORCEMENTS, AND/OR EQUIPMENT INSTALLATIONS SHALL NOT COMPROMISE THE INTEGRITY OR FUNCTIONAL USE OF THE SAFETY CLIMB OR ANY COMPONENTS OF THE CLIMBING FACILITY ON THE STRUCTURE. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO: PINCHING OF THE WIRE ROPE, BENDING OF THE WIRE ROPE FROM ITS SUPPORTS, DIRECT CONTACT OR CLOSE PROXIMITY TO THE WIRE ROPE WHICH MAY CAUSE FRICTIONAL WEAR, IMPACT TO THE ANCHORAGE POINTS IN ANY WAY, OR TO IMPEDE/BLOCK ITS INTENDED USE. ANY COMPROMISED SAFETY CLIMB, INCLUDING EXISTING CONDITIONS MUST BE TAGGED OUT AND REPORTED TO YOUR DISH Wireless L.L.C. AND DISH Wireless L.L.C. AND TOWER OWNER POC OR CALL THE NOC TO GENERATE A SAFETY CLIMB MAINTENANCE AND CONTRACTOR NOTICE TICKET.
3. PRIOR TO THE START OF CONSTRUCTION, ALL REQUIRED JURISDICTIONAL PERMITS SHALL BE OBTAINED. THIS INCLUDES, BUT IS NOT LIMITED TO, BUILDING, ELECTRICAL, MECHANICAL, FIRE, FLOOD ZONE, ENVIRONMENTAL, AND ZONING. AFTER ONSITE ACTIVITIES AND CONSTRUCTION ARE COMPLETED, ALL REQUIRED PERMITS SHALL BE SATISFIED AND CLOSED OUT ACCORDING TO LOCAL JURISDICTIONAL REQUIREMENTS.
4. ALL CONSTRUCTION MEANS AND METHODS; INCLUDING BUT NOT LIMITED TO, ERECTION PLANS, RIGGING PLANS, CLIMBING PLANS, AND RESCUE PLANS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR RESPONSIBLE FOR THE EXECUTION OF THE WORK CONTAINED HEREIN, AND SHALL MEET ANSI/ASSE A10.48 (LATEST EDITION); FEDERAL, STATE, AND LOCAL REGULATIONS; AND ANY APPLICABLE INDUSTRY CONSENSUS STANDARDS RELATED TO THE CONSTRUCTION ACTIVITIES BEING PERFORMED. ALL RIGGING PLANS SHALL ADHERE TO ANSI/ASSE A10.48 (LATEST EDITION) AND DISH Wireless L.L.C. AND TOWER OWNER STANDARDS, INCLUDING THE REQUIRED INVOLVEMENT OF A QUALIFIED ENGINEER FOR CLASS IV CONSTRUCTION, TO CERTIFY THE SUPPORTING STRUCTURE(S) IN ACCORDANCE WITH ANSI/TIA–322 (LATEST EDITION).
5. ALL SITE WORK TO COMPLY WITH DISH Wireless L.L.C. AND TOWER OWNER INSTALLATION STANDARDS FOR CONSTRUCTION ACTIVITIES ON DISH Wireless L.L.C. AND TOWER OWNER TOWER SITE AND LATEST VERSION OF ANSI/TIA–1019–A–2012 "STANDARD FOR INSTALLATION, ALTERATION, AND MAINTENANCE OF ANTENNA SUPPORTING STRUCTURES AND ANTENNAS."
6. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY DISH Wireless L.L.C. AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.
7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.
8. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER’S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.
9. THE CONTRACTOR SHALL CONTACT UTILITY LOCATING SERVICES INCLUDING PRIVATE LOCATES SERVICES PRIOR TO THE START OF CONSTRUCTION.
10. ALL EXISTING ACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES WHERE ENCOUNTERED IN THE WORK, SHALL BE PROTECTED AT ALL TIMES AND WHERE REQUIRED FOR THE PROPER EXECUTION OF THE WORK, SHALL BE RELOCATED AS DIRECTED BY CONTRACTOR. EXTREME CAUTION SHOULD BE USED BY THE CONTRACTOR WHEN EXCAVATING OR DRILLING PIERS AROUND OR NEAR UTILITIES. CONTRACTOR SHALL PROVIDE SAFETY TRAINING FOR THE WORKING CREW. THIS WILL INCLUDE BUT NOT BE LIMITED TO
A) FALL PROTECTION
B) CONFINED SPACE
C) ELECTRICAL SAFETY
D) TRENCHING AND EXCAVATION
E) CONSTRUCTION SAFETY PROCEDURES.
11. ALL SITE WORK SHALL BE AS INDICATED ON THE STAMPED CONSTRUCTION DRAWINGS AND DISH PROJECT SPECIFICATIONS, LATEST APPROVED REVISION.
12. CONTRACTOR SHALL KEEP THE SITE FREE FROM ACCUMULATING WASTE MATERIAL, DEBRIS, AND TRASH AT THE COMPLETION OF THE WORK. IF NECESSARY, RUBBISH, STUMPS, DEBRIS, STICKS, STONES AND OTHER REFUSE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF LEGALLY.
13. ALL EXISTING INACTIVE SEWER, WATER, GAS, ELECTRIC AND OTHER UTILITIES, WHICH INTERFERE WITH THE EXECUTION OF THE WORK, SHALL BE REMOVED AND/OR CAPPED, PLUGGED OR OTHERWISE DISCONTINUED AT POINTS WHICH WILL NOT INTERFERE WITH THE EXECUTION OF THE WORK, SUBJECT TO THE APPROVAL OF DISH Wireless L.L.C. AND TOWER OWNER, AND/OR LOCAL UTILITIES.
14. THE CONTRACTOR SHALL PROVIDE SITE SIGNAGE IN ACCORDANCE WITH THE TECHNICAL SPECIFICATION FOR SITE SIGNAGE REQUIRED BY LOCAL JURISDICTION AND SIGNAGE REQUIRED ON INDIVIDUAL PIECES OF EQUIPMENT, ROOMS, AND SHELTERS.
15. THE SITE SHALL BE GRADED TO CAUSE SURFACE WATER TO FLOW AWAY FROM THE CARRIER’S EQUIPMENT AND TOWER AREAS.
16. THE SUB GRADE SHALL BE COMPACTED AND BROUGHT TO A SMOOTH UNIFORM GRADE PRIOR TO FINISHED SURFACE APPLICATION.
17. THE AREAS OF THE OWNERS PROPERTY DISTURBED BY THE WORK AND NOT COVERED BY THE TOWER, EQUIPMENT OR DRIVEWAY, SHALL BE GRADED TO A UNIFORM SLOPE, AND STABILIZED TO PREVENT EROSION AS SPECIFIED ON THE CONSTRUCTION DRAWINGS AND/OR PROJECT SPECIFICATIONS.
18. CONTRACTOR SHALL MINIMIZE DISTURBANCE TO EXISTING SITE DURING CONSTRUCTION. EROSION CONTROL MEASURES, IF REQUIRED DURING CONSTRUCTION, SHALL BE IN CONFORMANCE WITH THE LOCAL GUIDELINES FOR EROSION AND SEDIMENT CONTROL.
19. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR’S EXPENSE TO THE SATISFACTION OF OWNER.
20. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS AND RADIOS REMOVED SHALL BE RETURNED TO THE OWNER’S DESIGNATED LOCATION.
21. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.
22. NO FILL OR EMBANKMENT MATERIAL SHALL BE PLACED ON FROZEN GROUND. FROZEN MATERIALS, SNOW OR ICE SHALL NOT

BE PLACED IN ANY FILL OR EMBANKMENT.

GENERAL NOTES:

1.FOR THE PURPOSE OF CONSTRUCTION DRAWING, THE FOLLOWING DEFINITIONS SHALL APPLY:

CONTRACTOR:GENERAL CONTRACTOR RESPONSIBLE FOR CONSTRUCTION

CARRIER:DISH Wireless L.L.C.

STRUCTURE OWNER:SEE T–1 FOR INFO

2. THESE DRAWINGS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS NORMALLY EXERCISED UNDER SIMILAR CIRCUMSTANCES BY REPUTABLE ENGINEERS IN THIS OR SIMILAR LOCALITIES. IT IS ASSUMED THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKPEOPLE WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANEOUS WORK NOT EXPLICITLY SHOWN.

3. THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY FOR PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, FORMWORK, SHORING, ETC. SITE VISITS BY THE ENGINEER OR HIS REPRESENTATIVE WILL NOT INCLUDE INSPECTION OF THESE ITEMS AND IS FOR STRUCTURAL OBSERVATION OF THE FINISHED STRUCTURE ONLY.

4. NOTES AND DETAILS IN THE CONSTRUCTION DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT, AND/OR AS PROVIDED FOR IN THE CONTRACT DOCUMENTS. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE GREATER, MORE STRICT REQUIREMENTS, SHALL GOVERN. IF FURTHER CLARIFICATION IS REQUIRED CONTACT THE ENGINEER OF RECORD.

5. SUBSTANTIAL EFFORT HAS BEEN MADE TO PROVIDE ACCURATE DIMENSIONS AND MEASUREMENTS ON THE DRAWINGS TO ASSIST IN THE FABRICATION AND/OR PLACEMENT OF CONSTRUCTION ELEMENTS BUT IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE DIMENSIONS, MEASUREMENTS, AND/OR CLEARANCES SHOWN IN THE CONSTRUCTION DRAWINGS PRIOR TO FABRICATION OR CUTTING OF ANY NEW OR EXISTING CONSTRUCTION ELEMENTS. IF IT IS DETERMINED THAT THERE ARE DISCREPANCIES AND/OR CONFLICTS WITH THE CONSTRUCTION DRAWINGS THE ENGINEER OF RECORD IS TO BE NOTIFIED AS SOON AS POSSIBLE.

6. PRIOR TO THE SUBMISSION OF BIDS, THE BIDDING CONTRACTOR SHALL VISIT THE CELL SITE TO FAMILIARIZE WITH THE EXISTING CONDITIONS AND TO CONFIRM THAT THE WORK CAN BE ACCOMPLISHED AS SHOWN ON THE CONSTRUCTION DRAWINGS. ANY DISCREPANCY FOUND SHALL BE BROUGHT TO THE ATTENTION OF CARRIER POC AND TOWER OWNER.

7. ALL MATERIALS FURNISHED AND INSTALLED SHALL BE IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, REGULATIONS AND ORDINANCES. CONTRACTOR SHALL ISSUE ALL APPROPRIATE NOTICES AND COMPLY WITH ALL LAWS, ORDINANCES, RULES, REGULATIONS AND LAWFUL ORDERS OF ANY PUBLIC AUTHORITY REGARDING THE PERFORMANCE OF THE WORK. ALL WORK CARRIED OUT SHALL COMPLY WITH ALL APPLICABLE MUNICIPAL AND UTILITY COMPANY SPECIFICATIONS AND LOCAL JURISDICTIONAL CODES, ORDINANCES AND APPLICABLE REGULATIONS.

8. UNLESS NOTED OTHERWISE, THE WORK SHALL INCLUDE FURNISHING MATERIALS, EQUIPMENT, APPURTENANCES AND LABOR NECESSARY TO COMPLETE ALL INSTALLATIONS AS INDICATED ON THE DRAWINGS.

9. THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT AND MATERIALS IN ACCORDANCE WITH MANUFACTURER’S RECOMMENDATIONS UNLESS SPECIFICALLY STATED OTHERWISE.

10. IF THE SPECIFIED EQUIPMENT CAN NOT BE INSTALLED AS SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL PROPOSE AN ALTERNATIVE INSTALLATION FOR APPROVAL BY THE CARRIER AND TOWER OWNER PRIOR TO PROCEEDING WITH ANY SUCH CHANGE OF INSTALLATION.

11. CONTRACTOR IS TO PERFORM A SITE INVESTIGATION, BEFORE SUBMITTING BIDS, TO DETERMINE THE BEST ROUTING OF ALL CONDUITS FOR POWER, AND TELCO AND FOR GROUNDING CABLES AS SHOWN IN THE POWER, TELCO, AND GROUNDING PLAN DRAWINGS.

12. THE CONTRACTOR SHALL PROTECT EXISTING IMPROVEMENTS, PAVEMENTS, CURBS, LANDSCAPING AND STRUCTURES. ANY DAMAGED PART SHALL BE REPAIRED AT CONTRACTOR’S EXPENSE TO THE SATISFACTION OF DISH Wireless L.L.C. AND TOWER OWNER

13. CONTRACTOR SHALL LEGALLY AND PROPERLY DISPOSE OF ALL SCRAP MATERIALS SUCH AS COAXIAL CABLES AND OTHER ITEMS REMOVED FROM THE EXISTING FACILITY. ANTENNAS REMOVED SHALL BE RETURNED TO THE OWNER’S DESIGNATED LOCATION.

14. CONTRACTOR SHALL LEAVE PREMISES IN CLEAN CONDITION. TRASH AND DEBRIS SHOULD BE REMOVED FROM SITE ON A DAILY BASIS.



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SHEET TITLE

GENERAL NOTES

SHEET NUMBER

GN-3

CONCRETE, FOUNDATIONS, AND REINFORCING STEEL:

1.

ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE ACI 301, ACI 318, ACI 336, ASTM A184, ASTM A185 AND THE DESIGN AND CONSTRUCTION SPECIFICATION FOR CAST-IN-PLACE CONCRETE.
2.

UNLESS NOTED OTHERWISE, SOIL BEARING PRESSURE USED FOR DESIGN OF SLABS AND FOUNDATIONS IS ASSUMED TO BE 1000 psf.
3.

ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'c) OF 3000 psi AT 28 DAYS, UNLESS NOTED OTHERWISE. NO MORE THAN 90 MINUTES SHALL ELAPSE FROM BATCH TIME TO TIME OF PLACEMENT UNLESS APPROVED BY THE ENGINEER OF RECORD. TEMPERATURE OF CONCRETE SHALL NOT EXCEED 90°F AT TIME OF PLACEMENT.
4.

CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL CONTAIN AIR ENTRAINING ADMIXTURES. AMOUNT OF AIR ENTRAINMENT TO BE BASED ON SIZE OF AGGREGATE AND F3 CLASS EXPOSURE (VERY SEVERE). CEMENT USED TO BE TYPE II PORTLAND CEMENT WITH A MAXIMUM WATER-TO-CEMENT RATIO (W/C) OF 0.45.
5.

ALL STEEL REINFORCING SHALL CONFORM TO ASTM A615. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185. ALL SPLICES SHALL BE CLASS "B" TENSION SPLICES, UNLESS NOTED OTHERWISE. ALL HOOKS SHALL BE STANDARD 90 DEGREE HOOKS, UNLESS NOTED OTHERWISE. YIELD STRENGTH (Fy) OF STANDARD DEFORMED BARS ARE AS FOLLOWS:
#4 BARS AND SMALLER 40 ksi
#5 BARS AND LARGER 60 ksi
6.

THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING STEEL UNLESS SHOWN OTHERWISE ON DRAWINGS:
 - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"
 - CONCRETE EXPOSED TO EARTH OR WEATHER:
 - #6 BARS AND LARGER 2"
 - #5 BARS AND SMALLER 1-1/2"
 - CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
 - SLAB AND WALLS 3/4"
 - BEAMS AND COLUMNS 1-1/2"
7.

A TOOLED EDGE OR A 3/4" CHAMFER SHALL BE PROVIDED AT ALL EXPOSED EDGES OF CONCRETE, UNLESS NOTED OTHERWISE, IN ACCORDANCE WITH ACI 301 SECTION 4.2.4.

ELECTRICAL INSTALLATION NOTES:

1.

ALL ELECTRICAL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, NEC AND ALL APPLICABLE FEDERAL, STATE, AND LOCAL CODES/ORDINANCES.
2.

CONDUIT ROUTINGS ARE SCHEMATIC. CONTRACTOR SHALL INSTALL CONDUITS SO THAT ACCESS TO EQUIPMENT IS NOT BLOCKED AND TRIP HAZARDS ARE ELIMINATED.
3.

WIRING, RACEWAY AND SUPPORT METHODS AND MATERIALS SHALL COMPLY WITH THE REQUIREMENTS OF THE NEC.
4.

ALL CIRCUITS SHALL BE SEGREGATED AND MAINTAIN MINIMUM CABLE SEPARATION AS REQUIRED BY THE NEC.
- 4.1.

ALL EQUIPMENT SHALL BEAR THE UNDERWRITERS LABORATORIES LABEL OF APPROVAL, AND SHALL CONFORM TO REQUIREMENT OF THE NATIONAL ELECTRICAL CODE.
- 4.2.

ALL OVERCURRENT DEVICES SHALL HAVE AN INTERRUPTING CURRENT RATING THAT SHALL BE GREATER THAN THE SHORT CIRCUIT CURRENT TO WHICH THEY ARE SUBJECTED, 22,000 AIC MINIMUM. VERIFY AVAILABLE SHORT CIRCUIT CURRENT DOES NOT EXCEED THE RATING OF ELECTRICAL EQUIPMENT IN ACCORDANCE WITH ARTICLE 110.24 NEC OR THE MOST CURRENT ADOPTED CODE PRE THE GOVERNING JURISDICTION.
5.

EACH END OF EVERY POWER PHASE CONDUCTOR, GROUNDING CONDUCTOR, AND TELCO CONDUCTOR OR CABLE SHALL BE LABELED WITH COLOR-CODED INSULATION OR ELECTRICAL TAPE (3M BRAND, 1/2" PLASTIC ELECTRICAL TAPE WITH UV PROTECTION, OR EQUAL). THE IDENTIFICATION METHOD SHALL CONFORM WITH NEC AND OSHA.
6.

ALL ELECTRICAL COMPONENTS SHALL BE CLEARLY LABELED WITH LAMICOID TAGS SHOWING THEIR RATED VOLTAGE, PHASE CONFIGURATION, WIRE CONFIGURATION, POWER OR AMPACITY RATING AND BRANCH CIRCUIT ID NUMBERS (i.e. PANEL BOARD AND CIRCUIT ID'S).
7.

PANEL BOARDS (ID NUMBERS) SHALL BE CLEARLY LABELED WITH PLASTIC LABELS.
8.

TIE WRAPS ARE NOT ALLOWED.
9.

ALL POWER AND EQUIPMENT GROUND WIRING IN TUBING OR CONDUIT SHALL BE SINGLE COPPER CONDUCTOR (#14 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
10.

SUPPLEMENTAL EQUIPMENT GROUND WIRING LOCATED INDOORS SHALL BE SINGLE COPPER CONDUCTOR (#6 OR LARGER) WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
11.

POWER AND CONTROL WIRING IN FLEXIBLE CORD SHALL BE MULTI-CONDUCTOR, TYPE SOOW CORD (#14 OR LARGER) UNLESS OTHERWISE SPECIFIED.
12.

POWER AND CONTROL WIRING FOR USE IN CABLE TRAY SHALL BE MULTI-CONDUCTOR, TYPE TC CABLE (#14 OR LARGER), WITH TYPE THHW, THWN, THWN-2, XHHW, XHHW-2, THW, THW-2, RHW, OR RHW-2 INSULATION UNLESS OTHERWISE SPECIFIED.
13.

ALL POWER AND GROUNDING CONNECTIONS SHALL BE CRIMP-STYLE, COMPRESSION WIRE LUGS AND WIRE NUTS BY THOMAS AND BETTS (OR EQUAL). LUGS AND WIRE NUTS SHALL BE RATED FOR OPERATION NOT LESS THAN 75° C (90° C IF AVAILABLE).
14.

RACEWAY AND CABLE TRAY SHALL BE LISTED OR LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND NEC.
15.

ELECTRICAL METALLIC TUBING (EMT), INTERMEDIATE METAL CONDUIT (IMC), OR RIGID METAL CONDUIT (RMC) SHALL BE USED FOR EXPOSED INDOOR LOCATIONS.

16.

ELECTRICAL METALLIC TUBING (EMT) OR METAL-CLAD CABLE (MC) SHALL BE USED FOR CONCEALED INDOOR LOCATIONS.
17.

SCHEDULE 40 PVC UNDERGROUND ON STRAIGHTS AND SCHEDULE 80 PVC FOR ALL ELBOWS/90s AND ALL APPROVED ABOVE GRADE PVC CONDUIT.
18.

LIQUID-TIGHT FLEXIBLE METALLIC CONDUIT (LIQUID-TITE FLEX) SHALL BE USED INDOORS AND OUTDOORS, WHERE VIBRATION OCCURS OR FLEXIBILITY IS NEEDED.
19.

CONDUIT AND TUBING FITTINGS SHALL BE THREADED OR COMPRESSION-TYPE AND APPROVED FOR THE LOCATION USED. SET SCREW FITTINGS ARE NOT ACCEPTABLE.
20.

CABINETS, BOXES AND WIRE WAYS SHALL BE LABELED FOR ELECTRICAL USE IN ACCORDANCE WITH NEMA, UL, ANSI/IEEE AND THE NEC.
21.

WIREWAYS SHALL BE METAL WITH AN ENAMEL FINISH AND INCLUDE A HINGED COVER, DESIGNED TO SWING OPEN DOWNWARDS (WIREMOLD SPECMATE WIREWAY).
22.

SLOTTED WIRING DUCT SHALL BE PVC AND INCLUDE COVER (PANDUIT TYPE E OR EQUAL).
23.

CONDUITS SHALL BE FASTENED SECURELY IN PLACE WITH APPROVED NON-PERFORATED STRAPS AND HANGERS. EXPLOSIVE DEVICES (i.e. POWDER-ACTUATED) FOR ATTACHING HANGERS TO STRUCTURE WILL NOT BE PERMITTED. CLOSELY FOLLOW THE LINES OF THE STRUCTURE, MAINTAIN CLOSE PROXIMITY TO THE STRUCTURE AND KEEP CONDUITS IN TIGHT ENVELOPES. CHANGES IN DIRECTION TO ROUTE AROUND OBSTACLES SHALL BE MADE WITH CONDUIT OUTLET BODIES. CONDUIT SHALL BE INSTALLED IN A NEAT AND WORKMANLIKE MANNER. PARALLEL AND PERPENDICULAR TO STRUCTURE WALL AND CEILING LINES. ALL CONDUIT SHALL BE FISHED TO CLEAR OBSTRUCTIONS. ENDS OF CONDUITS SHALL BE TEMPORARILY CAPPED FLUSH TO FINISH GRADE TO PREVENT CONCRETE, PLASTER OR DIRT FROM ENTERING. CONDUITS SHALL BE RIGIDLY CLAMPED TO BOXES BY GALVANIZED MALLEABLE IRON BUSHING ON INSIDE AND GALVANIZED MALLEABLE IRON LOCKNUT ON OUTSIDE AND INSIDE.
24.

EQUIPMENT CABINETS, TERMINAL BOXES, JUNCTION BOXES AND PULL BOXES SHALL BE GALVANIZED OR EPOXY-COATED SHEET STEEL. SHALL MEET OR EXCEED UL 50 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND NEMA 3 (OR BETTER) FOR EXTERIOR LOCATIONS.
25.

METAL RECEPTACLE, SWITCH AND DEVICE BOXES SHALL BE GALVANIZED, EPOXY-COATED OR NON-CORRODING; SHALL MEET OR EXCEED UL 514A AND NEMA OS 1 AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
26.

NONMETALLIC RECEPTACLE, SWITCH AND DEVICE BOXES SHALL MEET OR EXCEED NEMA OS 2 (NEWEST REVISION) AND BE RATED NEMA 1 (OR BETTER) FOR INTERIOR LOCATIONS AND WEATHER PROTECTED (WP OR BETTER) FOR EXTERIOR LOCATIONS.
27.

THE CONTRACTOR SHALL NOTIFY AND OBTAIN NECESSARY AUTHORIZATION FROM THE CARRIER AND/OR DISH Wireless L.L.C. AND TOWER OWNER BEFORE COMMENCING WORK ON THE AC POWER DISTRIBUTION PANELS.
28.

THE CONTRACTOR SHALL PROVIDE NECESSARY TAGGING ON THE BREAKERS, CABLES AND DISTRIBUTION PANELS IN ACCORDANCE WITH THE APPLICABLE CODES AND STANDARDS TO SAFEGUARD LIFE AND PROPERTY.
29.

INSTALL LAMICOID LABEL ON THE METER CENTER TO SHOW "DISH Wireless L.L.C.".
30.

ALL EMPTY/SPARE CONDUITS THAT ARE INSTALLED ARE TO HAVE A METERED MULE TAPE PULL CORD INSTALLED.



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A&E PROJECT NUMBER
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DISH Wireless L.L.C.
PROJECT INFORMATION

MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
GENERAL NOTES

SHEET NUMBER
GN-4

GROUNDING NOTES:

1. ALL GROUND ELECTRODE SYSTEMS (INCLUDING TELECOMMUNICATION, RADIO, LIGHTNING PROTECTION AND AC POWER GES'S) SHALL BE BONDED TOGETHER AT OR BELOW GRADE, BY TWO OR MORE COPPER BONDING CONDUCTORS IN ACCORDANCE WITH THE NEC.
2. THE CONTRACTOR SHALL PERFORM IEEE FALL-OF-POTENTIAL RESISTANCE TO EARTH TESTING (PER IEEE 1100 AND 81) FOR GROUND ELECTRODE SYSTEMS, THE CONTRACTOR SHALL FURNISH AND INSTALL SUPPLEMENTAL GROUND ELECTRODES AS NEEDED TO ACHIEVE A TEST RESULT OF 5 OHMS OR LESS.
3. THE CONTRACTOR IS RESPONSIBLE FOR PROPERLY SEQUENCING GROUNDING AND UNDERGROUND CONDUIT INSTALLATION AS TO PREVENT ANY LOSS OF CONTINUITY IN THE GROUNDING SYSTEM OR DAMAGE TO THE CONDUIT AND PROVIDE TESTING RESULTS.
4. METAL CONDUIT AND TRAY SHALL BE GROUNDED AND MADE ELECTRICALLY CONTINUOUS WITH LISTED BONDING FITTINGS OR BY BONDING ACROSS THE DISCONTINUITY WITH #6 COPPER WIRE UL APPROVED GROUNDING TYPE CONDUIT CLAMPS.
5. METAL RACEWAY SHALL NOT BE USED AS THE NEC REQUIRED EQUIPMENT GROUND CONDUCTOR. STRANDED COPPER CONDUCTORS WITH GREEN INSULATION, SIZED IN ACCORDANCE WITH THE NEC, SHALL BE FURNISHED AND INSTALLED WITH THE POWER CIRCUITS TO BTS EQUIPMENT.
6. EACH CABINET FRAME SHALL BE DIRECTLY CONNECTED TO THE MASTER GROUND BAR WITH GREEN INSULATED SUPPLEMENTAL EQUIPMENT GROUND WIRES, #6 STRANDED COPPER OR LARGER FOR INDOOR BTS; #2 BARE SOLID TINNED COPPER FOR OUTDOOR BTS.
7. CONNECTIONS TO THE GROUND BUS SHALL NOT BE DOUBLED UP OR STACKED BACK TO BACK CONNECTIONS ON OPPOSITE SIDE OF THE GROUND BUS ARE PERMITTED.
8. ALL EXTERIOR GROUND CONDUCTORS BETWEEN EQUIPMENT/GROUND BARS AND THE GROUND RING SHALL BE #2 SOLID TINNED COPPER UNLESS OTHERWISE INDICATED.
9. ALUMINUM CONDUCTOR OR COPPER CLAD STEEL CONDUCTOR SHALL NOT BE USED FOR GROUNDING CONNECTIONS.
10. USE OF 90° BENDS IN THE PROTECTION GROUNDING CONDUCTORS SHALL BE AVOIDED WHEN 45° BENDS CAN BE ADEQUATELY SUPPORTED.
11. EXOTHERMIC WELDS SHALL BE USED FOR ALL GROUNDING CONNECTIONS BELOW GRADE.
12. ALL GROUND CONNECTIONS ABOVE GRADE (INTERIOR AND EXTERIOR) SHALL BE FORMED USING HIGH PRESS CRIMPS.
13. COMPRESSION GROUND CONNECTIONS MAY BE REPLACED BY EXOTHERMIC WELD CONNECTIONS.
14. ICE BRIDGE BONDING CONDUCTORS SHALL BE EXOTHERMICALLY BONDED OR BOLTED TO THE BRIDGE AND THE TOWER GROUND BAR.
15. APPROVED ANTIOXIDANT COATINGS (i.e. CONDUCTIVE GEL OR PASTE) SHALL BE USED ON ALL COMPRESSION AND BOLTED GROUND CONNECTIONS.
16. ALL EXTERIOR GROUND CONNECTIONS SHALL BE COATED WITH A CORROSION RESISTANT MATERIAL.
17. MISCELLANEOUS ELECTRICAL AND NON-ELECTRICAL METAL BOXES, FRAMES AND SUPPORTS SHALL BE BONDED TO THE GROUND RING, IN ACCORDANCE WITH THE NEC.
18. BOND ALL METALLIC OBJECTS WITHIN 6 ft OF MAIN GROUND RING WITH (1) #2 BARE SOLID TINNED COPPER GROUND CONDUCTOR.
19. GROUND CONDUCTORS USED FOR THE FACILITY GROUNDING AND LIGHTNING PROTECTION SYSTEMS SHALL NOT BE ROUTED THROUGH METALLIC OBJECTS THAT FORM A RING AROUND THE CONDUCTOR, SUCH AS METALLIC CONDUITS, METAL SUPPORT CLIPS OR SLEEVES THROUGH WALLS OR FLOORS. WHEN IT IS REQUIRED TO BE HOUSED IN CONDUIT TO MEET CODE REQUIREMENTS OR LOCAL CONDITIONS, NON-METALLIC MATERIAL SUCH AS PVC CONDUIT SHALL BE USED. WHERE USE OF METAL CONDUIT IS UNAVOIDABLE (i.e., NONMETALLIC CONDUIT PROHIBITED BY LOCAL CODE) THE GROUND CONDUCTOR SHALL BE BONDED TO EACH END OF THE METAL CONDUIT.
20. ALL GROUNDS THAT TRANSITION FROM BELOW GRADE TO ABOVE GRADE MUST BE #2 BARE SOLID TINNED COPPER IN 3/4" NON-METALLIC, FLEXIBLE CONDUIT FROM 24" BELOW GRADE TO WITHIN 3" TO 6" OF CAD-WELD TERMINATION POINT. THE EXPOSED END OF THE CONDUIT MUST BE SEALED WITH SILICONE CAULK. (ADD TRANSITIONING GROUND STANDARD DETAIL AS WELL).
21. BUILDINGS WHERE THE MAIN GROUNDING CONDUCTORS ARE REQUIRED TO BE ROUTED TO GRADE, THE CONTRACTOR SHALL ROUTE TWO GROUNDING CONDUCTORS FROM THE ROOFTOP, TOWERS, AND WATER TOWERS GROUNDING RING, TO THE EXISTING GROUNDING SYSTEM, THE GROUNDING CONDUCTORS SHALL NOT BE SMALLER THAN 2/0 COPPER. ROOFTOP GROUNDING RING SHALL BE BONDED TO THE EXISTING GROUNDING SYSTEM, THE BUILDING STEEL COLUMNS, LIGHTNING PROTECTION SYSTEM, AND BUILDING MAIN WATER LINE (FERROUS OR NONFERROUS METAL PIPING ONLY). DO NOT ATTACH GROUNDING TO FIRE SPRINKLER SYSTEM PIPES.



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2021.0030.0004

DISH Wireless L.L.C.
PROJECT INFORMATION

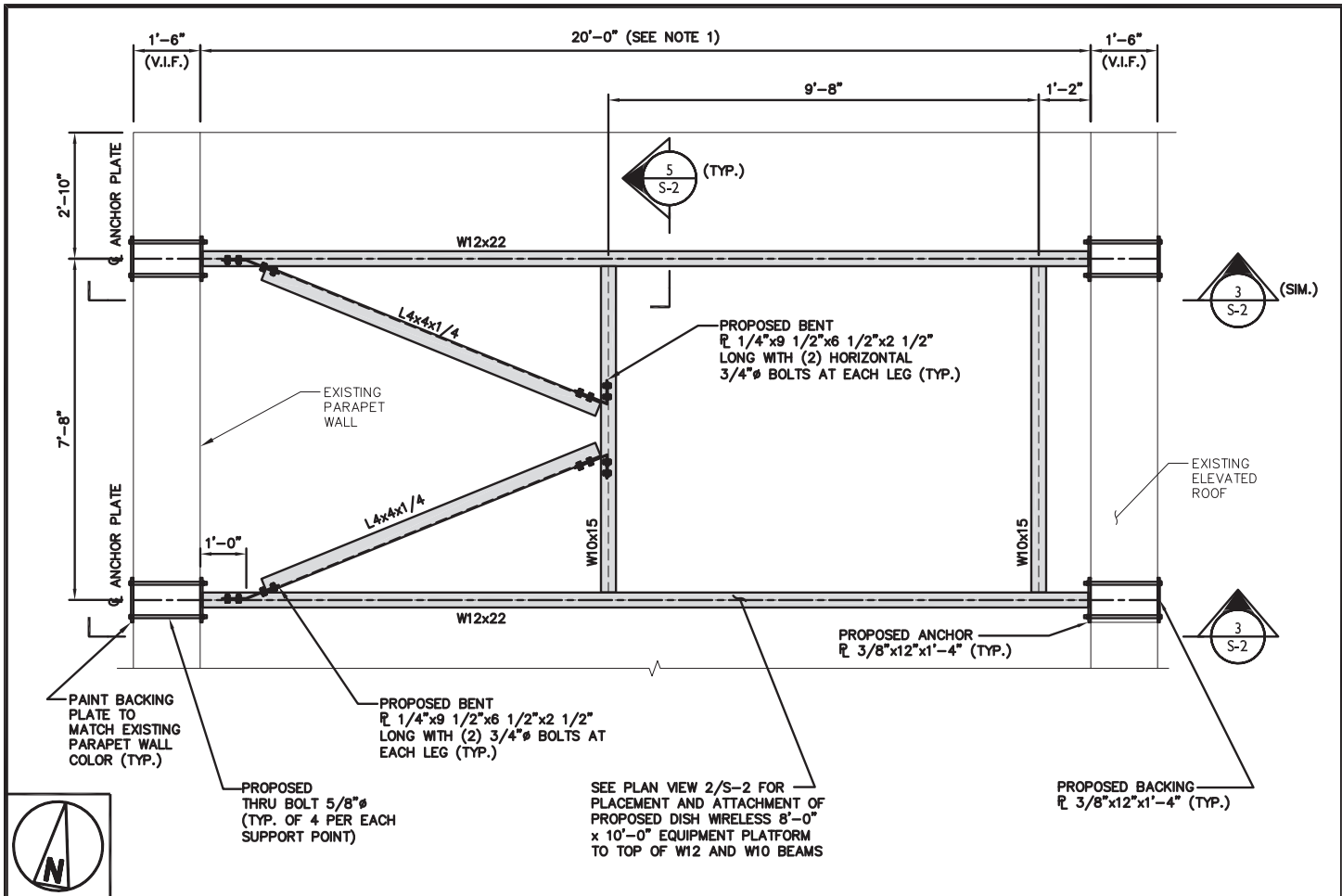
MWMKE00116B
314 6TH STREET
RACINE, WI 53403

SHEET TITLE
GENERAL NOTES

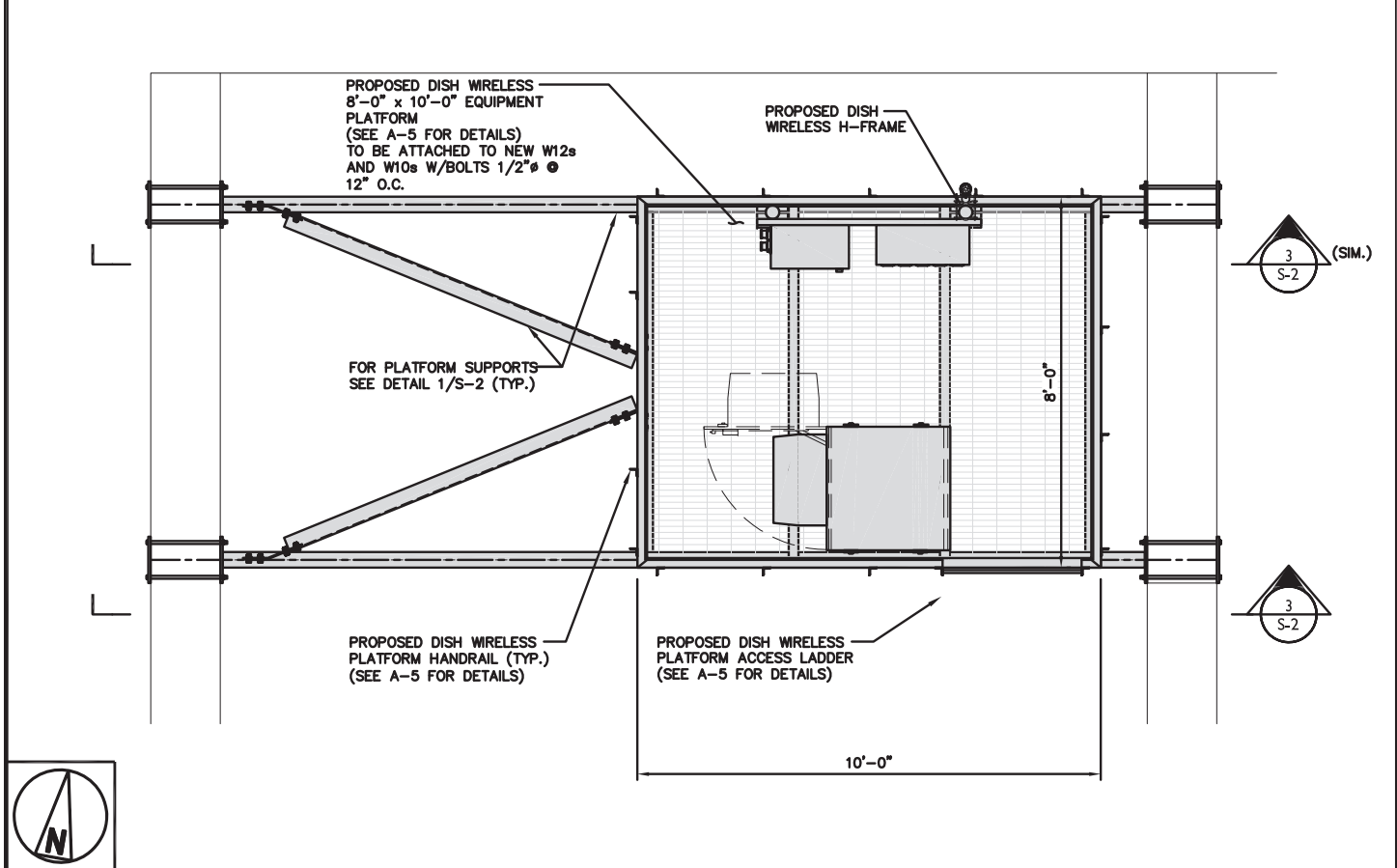
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GN-5

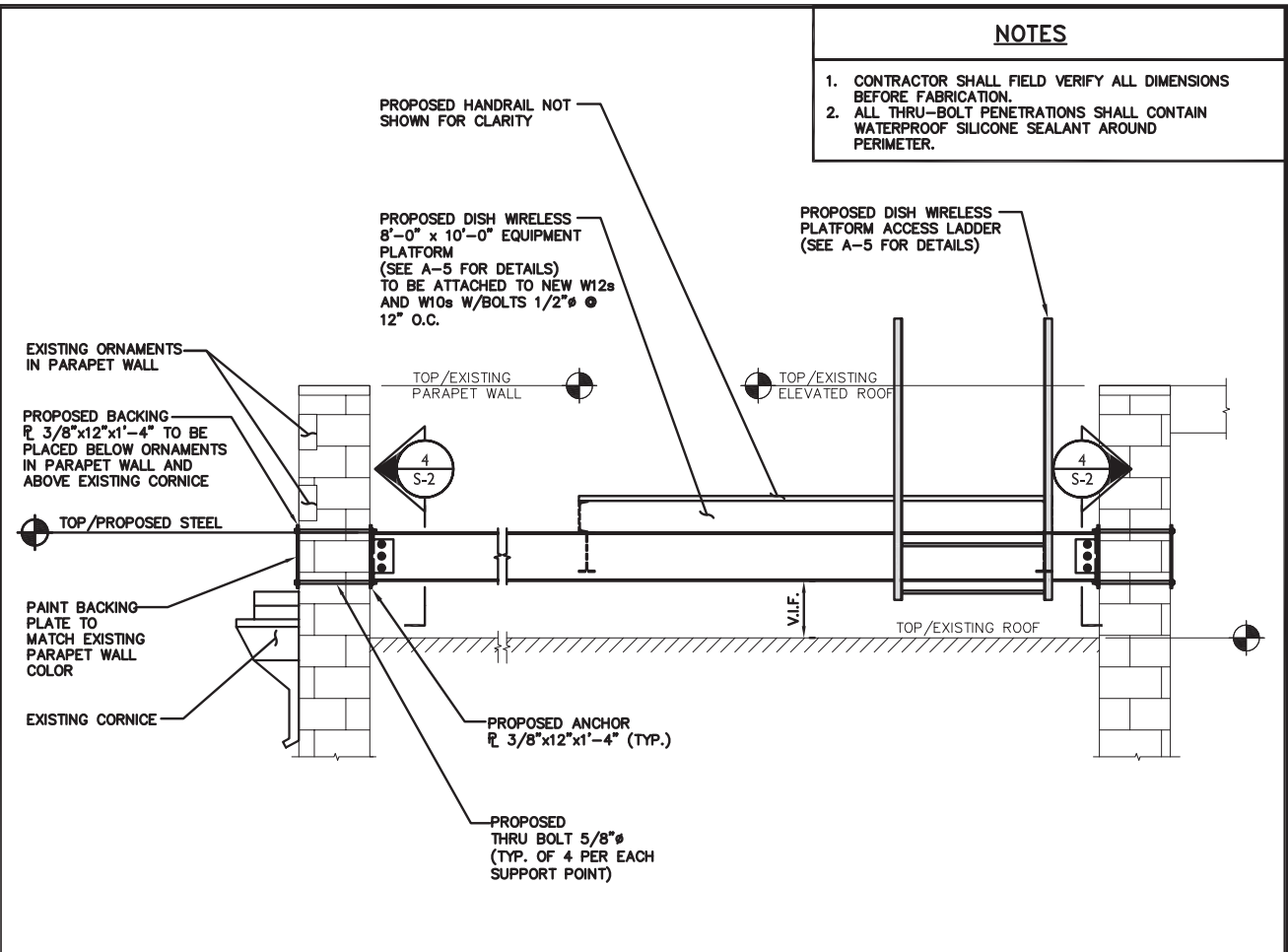
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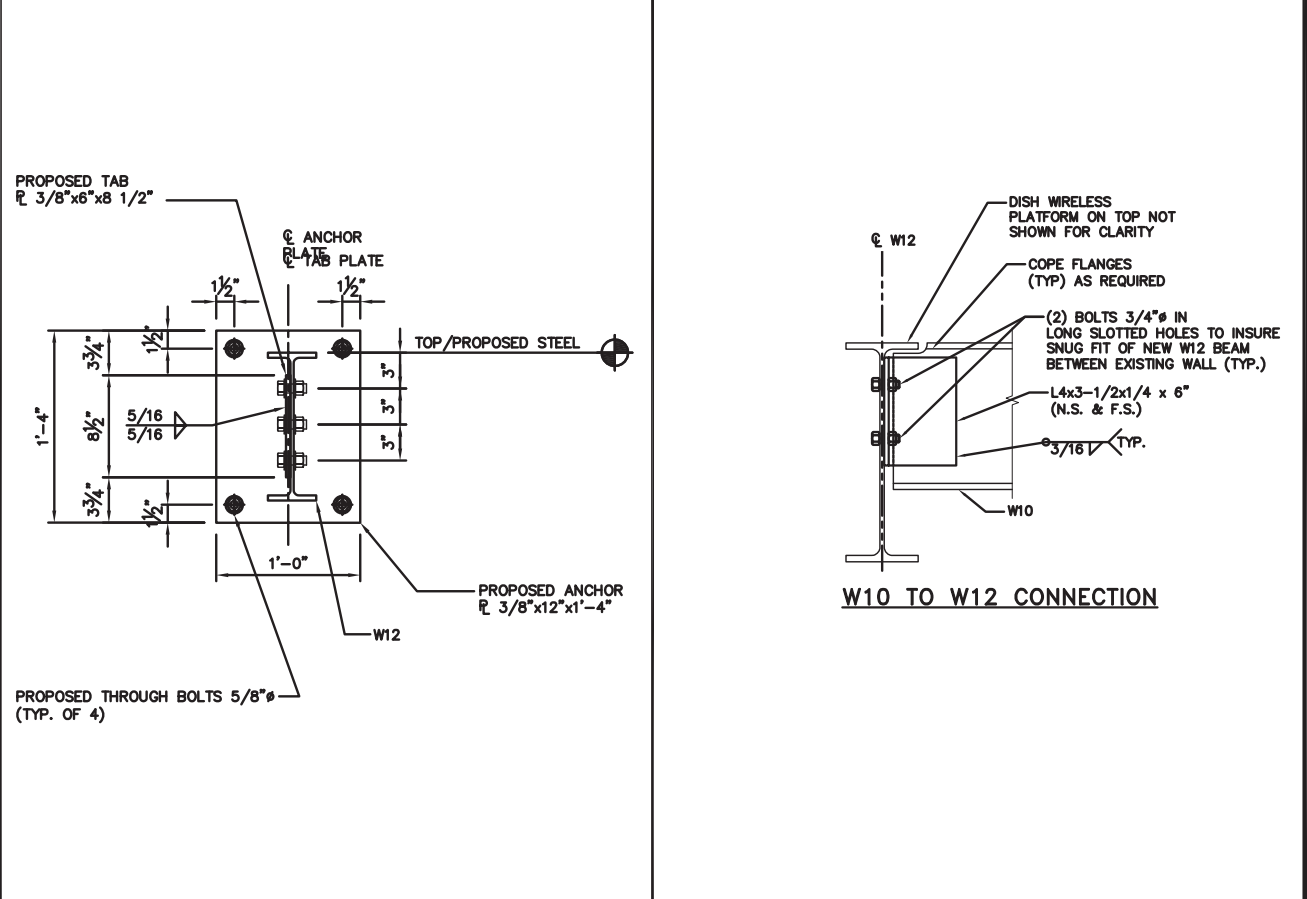
EQUIPMENT PLATFORM SUPPORT FRAMING PLAN



EQUIPMENT PLATFORM - PLAN VIEW



SECTION



TYP. SECTION

- NOTES
1. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS BEFORE FABRICATION.
 2. ALL THRU-BOLT PENETRATIONS SHALL CONTAIN WATERPROOF SILICONE SEALANT AROUND PERIMETER.



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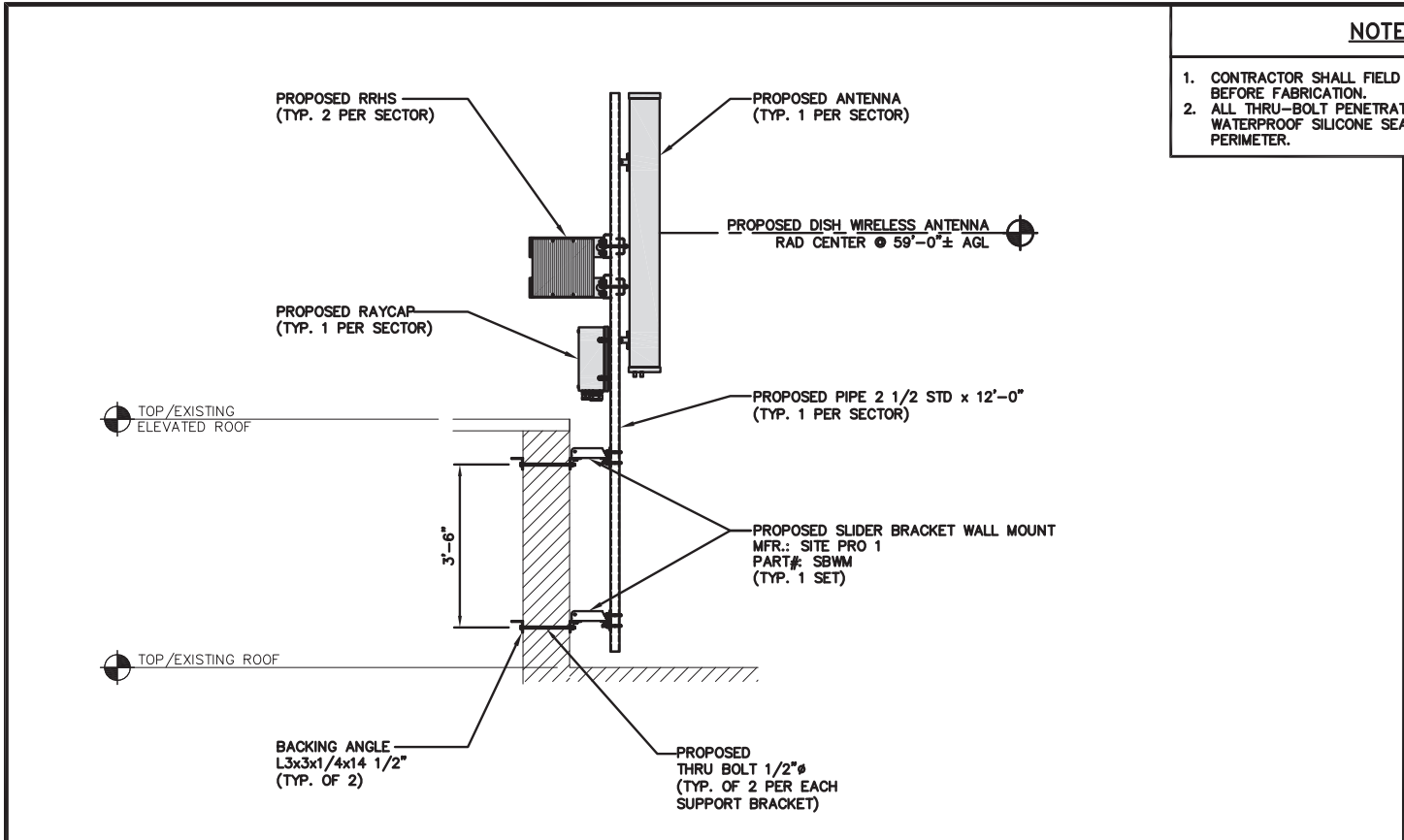
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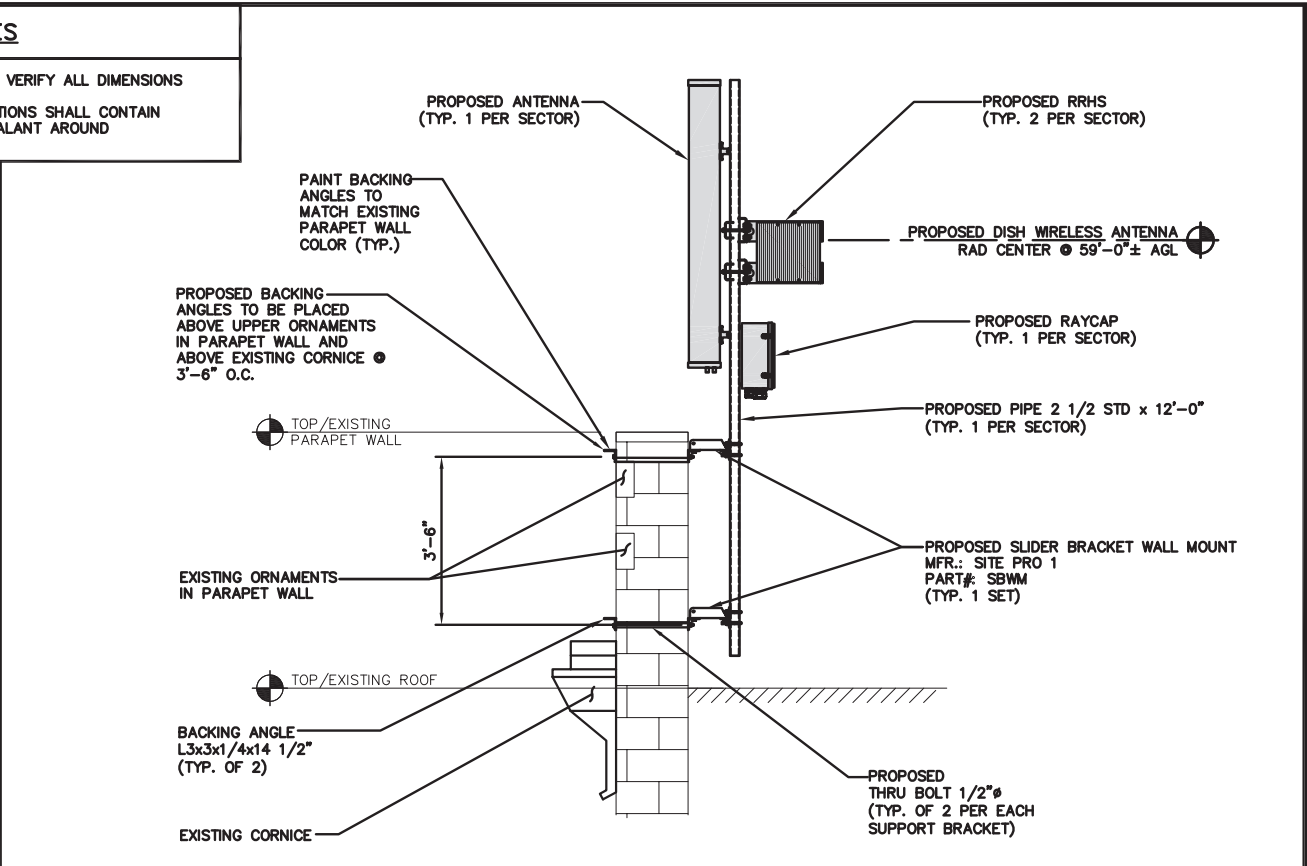
DISH Wireless L.L.C.
PROJECT INFORMATION
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SHEET TITLE
STRUCTURAL
DETAILS

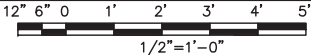
SHEET NUMBER
S-2



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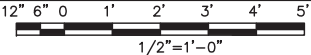


ANTENNA MOUNT SECTOR ALPHA

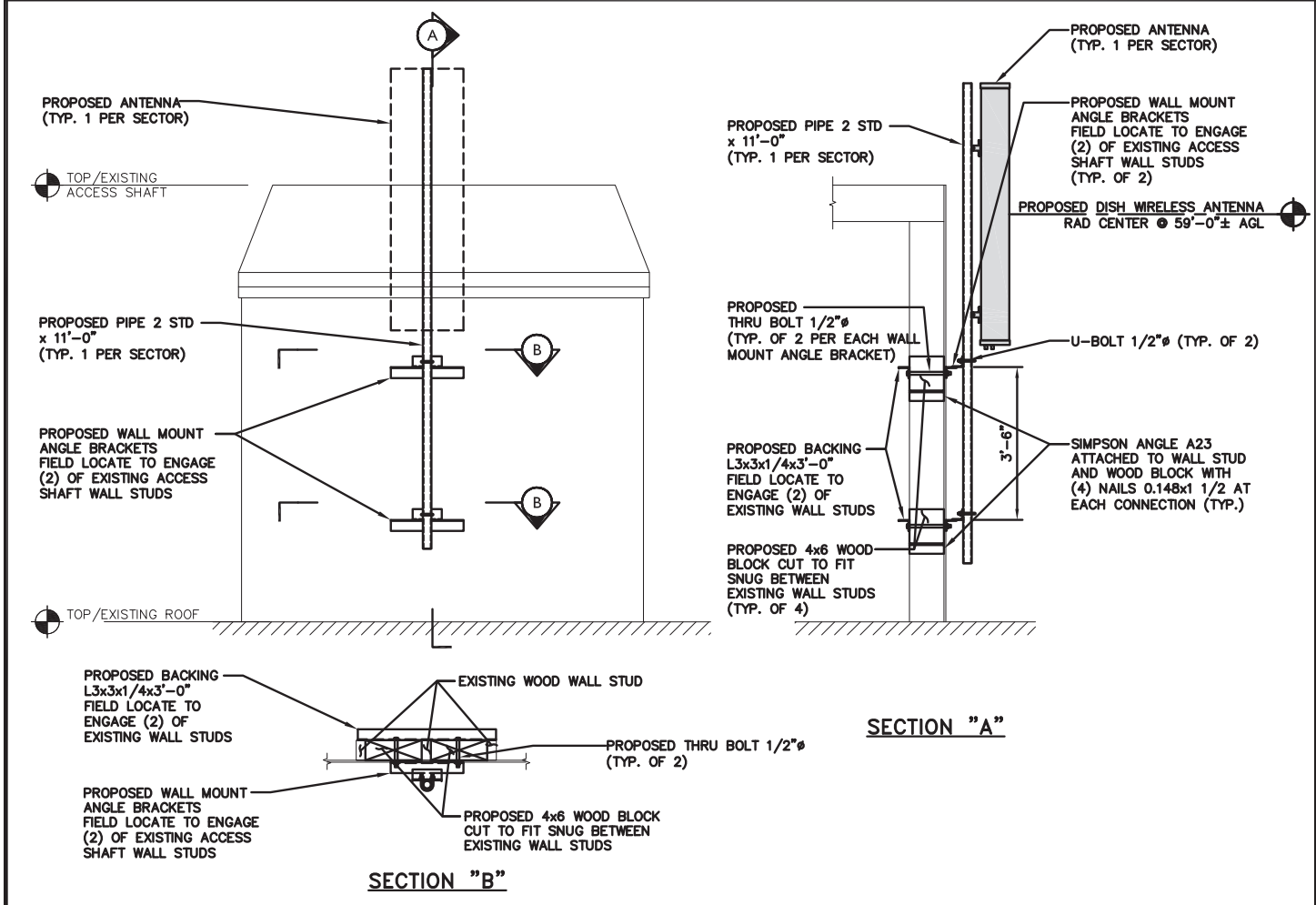


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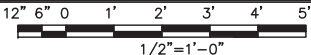
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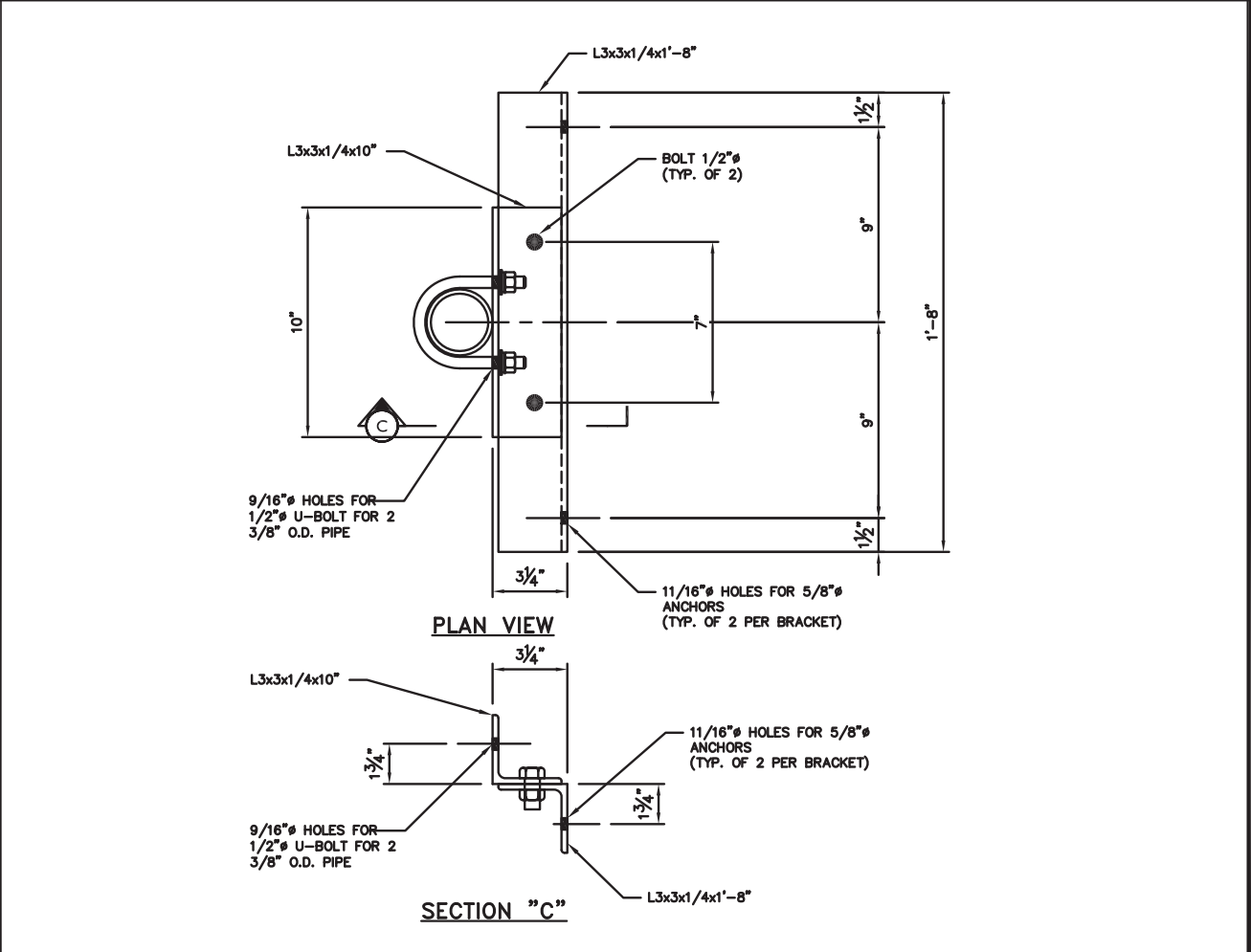
2



ANTENNA MOUNT SECTOR BETA



3



WALL MOUNT ANGLE BRACKET ASSEMBLY DETAIL

NO SCALE

4

dish
wireless.

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S-3



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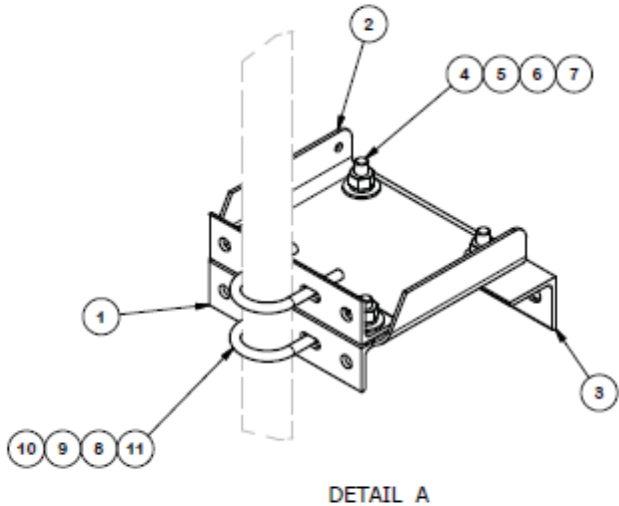
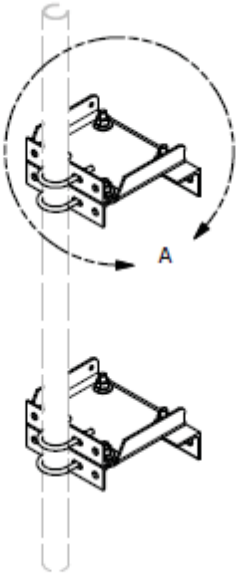
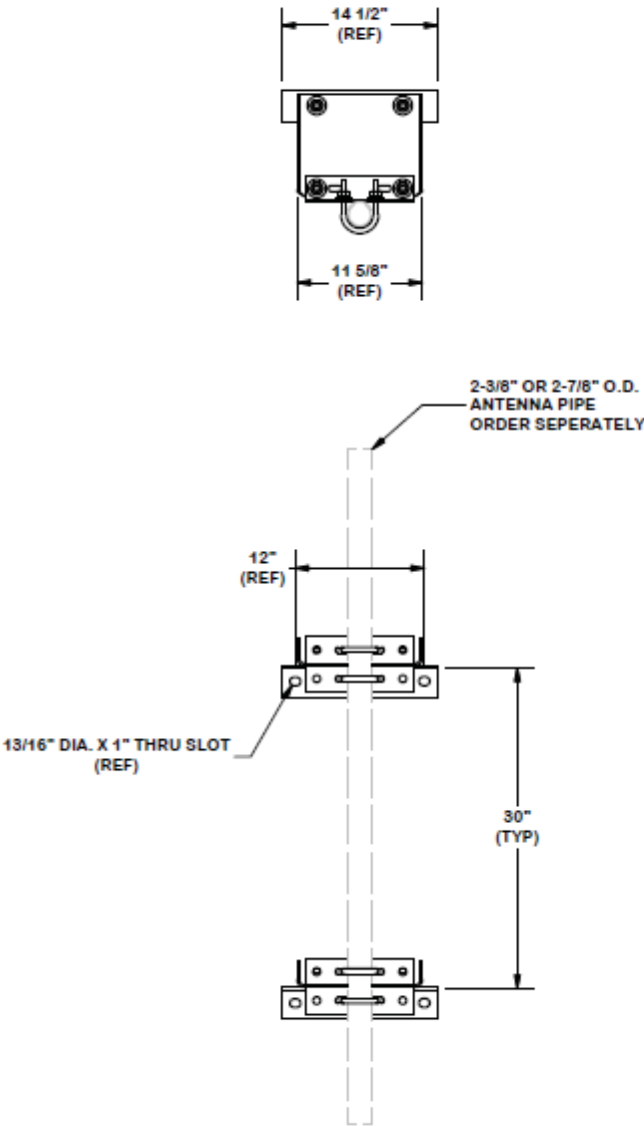
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SHEET TITLE
MOUNTING
SPECIFICATIONS

SHEET NUMBER
S-4



PARTS LIST						
ITEM	QTY	PART NO.	PART DESCRIPTION	LENGTH	UNIT WT.	NET WT.
1	4	X-SLD-A	SLIDER BRACKET ANGLE	10 in	3.28	13.12
2	2	X-SLD-BP	SLIDER BRACKET BENT PLATE	15 3/4 in	8.13	16.26
3	2	X-SLD3	SLIDER BRACKET WALL ANGLE	14 1/2 in	6.10	12.20
4	8	G5802	5/8" x 2" HDG HEX BOLT GR5		0.27	2.16
5	8	G58FW	5/8" HDG USS FLATWASHER	1/8 in	0.07	0.56
6	8	G58LW	5/8" HDG LOCKWASHER		0.03	0.21
7	8	G58NUT	5/8" HDG HEAVY 2H HEX NUT		0.13	1.04
8	8	G12FW	1/2" HDG USS FLATWASHER	3/32 in	0.03	0.27
9	8	G12LW	1/2" HDG LOCKWASHER	1/8 in	0.01	0.11
10	8	G12NUT	1/2" HDG HEAVY 2H HEX NUT		0.07	0.57
11	4	X-UB1212	1/2" X 2-1/2" X 4-1/2" X 2" GALV. U-BOLT		0.66	2.65
11	4	X-UB1300	1/2" X 3" X 5" X 2" GALV U-BOLT		0.74	2.95
TOTAL WT. #						51.93

TOLERANCE NOTES

TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE:
SAWED, SHEARED AND GAS CUT EDGES ($\pm 0.030"$)
DRILLED AND GAS CUT HOLES ($\pm 0.030"$) - NO CONING OF HOLES
LASER CUT EDGES AND HOLES ($\pm 0.010"$) - NO CONING OF HOLES
BENDS ARE $\pm 1/2$ DEGREE
ALL OTHER MACHINING ($\pm 0.030"$)
ALL OTHER ASSEMBLY ($\pm 0.060"$)

PROPRIETARY NOTE:
THE DATA AND TECHNIQUES CONTAINED IN THIS DRAWING ARE PROPRIETARY INFORMATION OF VALMONT INDUSTRIES AND CONSIDERED A TRADE SECRET. ANY USE OR DISCLOSURE WITHOUT THE CONSENT OF VALMONT INDUSTRIES IS STRICTLY PROHIBITED.

DESCRIPTION
SLIDER BRACKET
WALL MOUNT
2-3/8" & 2-7/8" O.D. PIPE



Locations:
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CLASS	SUB	DRAWING USAGE
81	01	CUSTOMER

PART NO.	SBWM
DWG. NO.	SBWM

PAGE
1 OF 1

STRUCTURAL ANALYSIS REPORT

STRUCTURE: ROOFTOP

PREPARED FOR: DISH Wireless

CARRIER: DISH Wireless

SITE NUMBER : MWMKE00116B

SITE LOCATION:
314 6th Street
Racine, WI 53403
N42.72684, W87.784674

DATE: June 2, 2022

RESULTS

PASS (MAX STRESS RATIO: 64.9%)



Jul 7 2022

Barbara T. Kotecki, P.E.

Summary

The structural analysis was performed by Fullerton, as requested by the client, to determine the conformance of existing structure with the governing 2018 Wisconsin Commercial Building Code and the industry standard, TIA-222-H (Structural Standards for Steel Antenna Supporting Structures, Antennas, and Small Wind Turbine Support Structures). The analysis considers the relevant structural properties, the existing and proposed appurtenances, and the required loading criteria.

Conclusion

Component	% Capacity	Pass / Fail
New Wall Mounts (Sector Alpha)	56.1%	PASS
New Wall Mounts (Sector Beta)	64.9%	PASS
New Wall Mounts (Sector Gamma)	51.6%	PASS
New Equipment Platform (Sector Gamma)	36.5%	PASS
Structural Rating (max from all components) = 64.9%		PASS

Sources

The following documents for the existing structure were made available for our structural analysis.

Reference Document	Date
RFDS Revision 2 provided by Dish	08/31/2021
Construction Drawings by Fullerton Engineering Consultants	Latest Version
Field Investigation Report by Fullerton Engineering	10/04/2021
Site Visit Photos	10/01/2021, 12/13/2021

Appurtenance Loading Schedule

EQUIPMENT		
ELEVATION (Ft)	QTY.	MANUFACTURER/MODEL
59'-0"	1	(N) Commscope FFVV-65B-R2
	1	(N) Fujitsu TA08025-B605 RRH Unit
	1	(N) Fujitsu TA08025-B604 RRH Unit
	1	(N) Raycap RDIDC-3045-PF-48 OVP
Rooftop	1	(N) Charles HEX CUBE-PM639155N4 (74"H x 32"W x 32"D – 1067.14#)
	1	(N) Raycap PPC RDIAC-2465-P-240-MTS
	1	(N) Charles CFIT-PF2020DSH1 Fiber Telco Enclosure

(E) denotes existing loading

(N) denotes proposed loading

- New appurtenances will be mounted on new wall mounts (Site Pro 1 Part# SBWM) in Sector Alpha and Gamma and new wall mount angles on Sector Beta.
- New equipment cabinet will be installed on new steel equipment platform attached to existing parapet wall and elevated roof wall..

Assumptions

This analysis is based on the theoretical capacity of the members and is not a condition assessment of the tower. The analysis is based solely on the information supplied, and the results, in turn, are only as accurate as data extracted from this information. Fullerton has been instructed by the client to assume the information supplied is accurate, and Fullerton has made no independent determination of its accuracy. The exception to the previous statement is if Fullerton has been contracted by the client to provide an independent structural mapping report of the tower and related appurtenances, in which case Fullerton has made an independent determination of the accuracy of the information resulting from the mapping report.

- Member sizes and geometry are considered accurate as supplied. The material grade is as per data supplied and/or as assumed and stated in the materials section.
- The existing structure is assumed to have been properly maintained in accordance with the TIA/EIA standard and/or its original manufacturer's recommendations. The existing structure is assumed to be in good condition with no structural defects and with no deterioration to its member capacities.
- The antenna configuration is as supplied and/or stated in the analysis section. It is assumed to be complete and accurate. All antennas, mounts, remote radios, cables, and cable supports are assumed to be properly installed and supported as per the manufacturer's requirements.
- The antennas, mounts, remote radios, cables, and cable supports, and lines stated in the appurtenance loading schedule represent Fullerton's understanding of the overall antenna configuration. If the actual configuration is different than above, then this analysis is invalid. Please refer to this report for the projected wind areas used in the calculations for antennas and mounts. If variations or discrepancies are identified, please inform Fullerton.
- Some assumptions are made regarding antenna and mount sizes and their projected areas based on a best interpretation of the data supplied and a best knowledge of antenna type and industry practice.
- All welds and connections are assumed to develop at least the member capacity, unless determined otherwise and explicitly stated in this report.
- All prior structural modifications, if any, are assumed to be as per data supplied/ available, to be properly installed and to be fully effective.

Scope and Limitations

The engineering services rendered by Fullerton Engineering Consultants, LLC (Fullerton) in connection with this structural analysis are limited to an analysis of the structure, size, and capacity of its members. Fullerton does not analyze the fabrication, including welding and connection capacities, except as included in this report.

The information and conclusions contained in this report were determined by application of the current engineering standards and analysis procedures and formulae, and Fullerton assumes no obligation to revise any of the information or conclusions contained in this report in the event such engineering and analysis procedures and formulae are hereafter modified or revised.

Fullerton makes no warranties, expressed, or implied in connection with this report and disclaims any liability arising from original design, material, fabrication, and erection deficiencies or the “as-built” condition of this tower.

Installation procedures are not within the scope of this report and should be performed and evaluated by a competent tower erection contractor.

Structural Calculations

Analysis and Design Criteria

Type of Structure	Rooftop	
Elevation of Antenna Centerline Above Ground	z := 59ft	
Structure Height Above Grade	h := 49.25ft	
Mid-Point of Structure	$h_{mid} := \frac{h}{2}$	
Mean Elevation of Structure above Sea Level	Z<sub>s</sub> := 624.28 ft	
Risk Category	II	ANSI/TIA-222-H: Table 2-1
Basic Wind Speed (no ice): 3-Second Gust	V := 107 mph	ANSI/TIA-222-H: ANNEX B
Basic Wind Speed (with ice): 3-Second Gust	V<sub>i</sub> := 40 mph	ANSI/TIA-222-H: ANNEX B
Basic Wind Speed (maintenance): 3-Second Gust	V<sub>m</sub> := 30 mph	ANSI/TIA-222-H: Section 16.3
Ice Thickness	t<sub>i</sub> := 1.5in	ANSI/TIA-222-H: ANNEX B
Exposure Category	C	ANSI/TIA-222-H: Section 2.6.5.1.2
Topographic Category	1	ANSI/TIA-222-H: Section 2.6.6.2.1
Height of Crest Above Surrounding Terrain	H := 5ft	
Gust Effect Factor	G<sub>h</sub> := 1	ANSI/TIA-222-H, Section 16.6



Ground Elevation Factor	$K_e = 0.98$	ANSI/TIA-222-H: Section 2.6.8
Wind Direction Probability Factor	$K_d = 0.95$	ANSI/TIA-222-H: Table 2-2
Velocity Pressure Coefficient	$K_z = 1.13$	ANSI/TIA-222-H: Section 2.6.5.2
Topographic Factor	$K_{zt} = 1$	ANSI/TIA-222-H: Section 2.6.6.2.1
Rooftop Wind Speed-Up Factor	$K_s = 1$	ANSI/TIA-222-H: Section 2.6.7
$q_z := 0.00256 \cdot K_z \cdot K_{zt} \cdot K_s \cdot K_e \cdot K_d \cdot V^2 \cdot \text{psf}$	$q_z = 30.83 \cdot \text{psf}$	Velocity Pressure ANSI/TIA-222-H: Section 2.6.11.6
$q_{zi} := 0.00256 \cdot K_z \cdot K_{zt} \cdot K_s \cdot K_e \cdot K_d \cdot V_i^2 \cdot \text{psf}$	$q_{zi} = 4.31 \cdot \text{psf}$	Velocity Pressure ANSI/TIA-222-H: Section 2.6.11.6
$q_{zm} := 0.00256 \cdot K_z \cdot K_{zt} \cdot K_s \cdot K_e \cdot K_d \cdot V_m^2 \cdot \text{psf}$	$q_{zm} = 2.42 \cdot \text{psf}$	Velocity Pressure ANSI/TIA-222-H: Section 2.6.11.6

Site Name: MWMKE00116
Site No.: MWMKE00116
Prepared By: AA
Checked By: BK

Fullerton Engineering Consultants, LLC.

Date: 6/2/2022

SITE INFORMATION; Seismic Lateral Design



ASCE 7 Hazards Report

Address:

No Address at This
Location

Standard:

ASCE/SEI 7-16

Risk Category: II

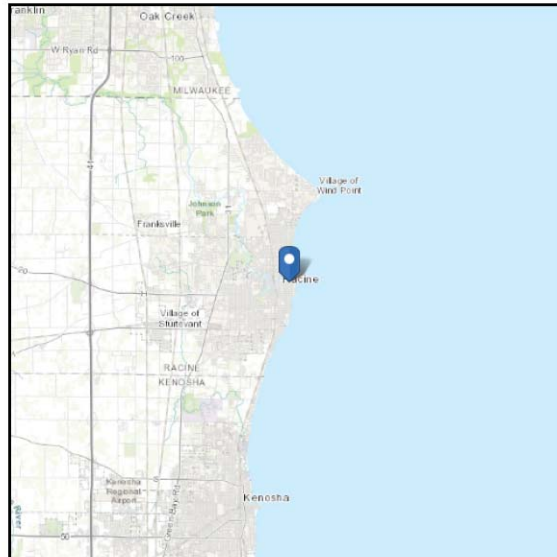
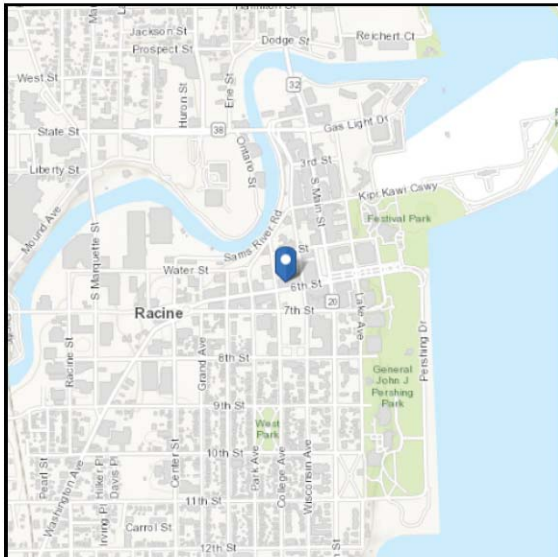
Soil Class:

D - Default (see
Section 11.4.3)

Elevation: 624.28 ft (NAVD 88)

Latitude: 42.72684

Longitude: -87.784674



Seismic

Site Soil Class:

D - Default (see Section 11.4.3)

Results:

S_S	0.084	S_{D1}	0.082
S_1	0.051	T_L	12
F_a	1.6	PGA	0.041
F_v	2.4	PGA _M	0.065
S_{MS}	0.134	F_{PGA}	1.6
S_{M1}	0.123	I_e	1
S_{DS}	0.089	C_v	0.7

Seismic Design Category

B

Importance Factor (Earthquake)

$$I_e := 1.0$$

ANSI/TIA-222-H: Table 2-3

Site Class

D - Stiff Soil (Default)



ANSI/TIA-222-H: Table 2-10

Seismic Design Category

B

MCE.R Ground Motion (period=0.2s)

$$S_S := 0.084$$

MCE.R Ground Motion (period=1.0s)

$$S_1 := 0.051$$

Seismic Design Value at 0.2s

$$S_{DS} := 0.089$$



Long-Period Site Coefficient, F_v

$$F_v = 2.4$$

ANSI/TIA-222-H, Table 2-12

Seismic Design Value at 1.0s

$$S_{D1} := 0.076$$

$$S_{D1} = 0.076$$

Long-period Transition Period (s)

$$T_L := 12$$

Response Modification Coefficient

$$R := 2$$

ANSI/TIA-222-H, 16.7

$$C_{scalc} := \frac{(S_{DS} \cdot I_e)}{R}$$

$$C_{scalc} = 0.04$$

Seismic Response Coefficient
 ANSI/TIA-222-H: 2.7.7.1.1

$$C_{smin1} := 0.044 \cdot S_{DS} \cdot I_e$$

$$C_{smin1} = 0$$

$$C_{smin2} := 0.03$$

$$C_{smin2} = 0.03$$

$$C_{smin3} := \begin{cases} \frac{(0.8 \cdot S_1 \cdot I_e)}{R} & \text{if } S_1 \geq 0.6 \\ C_{scalc} & \text{otherwise} \end{cases}$$

$$C_{smin3} = 0.04$$

$$C_s := \max(C_{scalc}, C_{smin1}, C_{smin2}, C_{smin3})$$

$$C_s = 0.04$$

$$A_{s1} := \begin{cases} 1 & \text{if } z \geq h_{mid} \\ \frac{(3 \cdot h_{mid}) - (2 \cdot z)}{h_{mid}} & \text{otherwise} \end{cases}$$

$$A_{s1} = 1$$

Amplification Factor
 ANSI/TIA-222-H: 2.7.8.1 and 16.7

$$A_s := \begin{cases} A_{s1} & \text{if Structure} = 1 \\ A_{s1} & \text{if Structure} = 2 \\ A_{s1} & \text{if Structure} = 3 \\ 3 & \text{if Structure} = 4 \\ 3 & \text{if Structure} = 5 \end{cases}$$

$$A_s = 3$$

New Commscope FFVV-65B-R2 Antenna

Height

height := 72in

Width/Diameter

width := 19.6in

Depth/Diameter

depth := 7.8in

Weight

weight := 98.1lbf

Shape

☒ Flat
☐ Round



$DL_1 := \text{weight}$

$$\frac{DL_1}{2} = 49.05 \text{ lbf}$$

Dead Load

$F_{F1} := F_F$

$$\frac{F_{F1}}{2} = 170.24 \text{ lbf}$$

Wind Load on Front Face

$F_{S1} := F_S$

$$\frac{F_{S1}}{2} = 79.77 \text{ lbf}$$

Wind Load on Side Face

$DL_{1i} := DL_{ice}$

$$\frac{DL_{1i}}{2} = 67.71 \text{ lbf}$$

Dead Load Ice

$F_{F1i} := F_{Fi}$

$$\frac{F_{F1i}}{2} = 28.49 \text{ lbf}$$

Wind Load with Ice on Front Face

$F_{S1i} := F_{Si}$

$$\frac{F_{S1i}}{2} = 15.48 \text{ lbf}$$

Wind Load with Ice on Side Face

$F_{F1m} := F_{Fm}$

$$\frac{F_{F1m}}{2} = 13.46 \text{ lbf}$$

Wind Load (maintenance) on Front Face

$F_{S1m} := F_{Sm}$

$$\frac{F_{S1m}}{2} = 6.33 \text{ lbf}$$

Wind Load (maintenance) on Side Face

$F_{Eh1} := \text{weight} \cdot A_s \cdot C_s$

$$\frac{F_{Eh1}}{2} = 6.55 \text{ lbf}$$

Lateral Earthquake Load

$F_{Ev1} := 0.2 \cdot S_{DS} \cdot \text{weight}$

$$\frac{F_{Ev1}}{2} = 0.87 \text{ lbf}$$

Vertical Earthquake Load

New Fujitsu TA08025-B605 Radio

Height

height := 15.75in

Width/Diameter

width := 14.96in

Depth/Diameter

depth := 9.06in

Weight

weight := 74.95lbf

Shape

☒ Flat
☐ Round



$DL_2 := \text{weight}$

$$\frac{DL_2}{2} = 37.48 \text{ lbf}$$

Dead Load

$F_{F2} := F_F$

$$\frac{F_{F2}}{2} = 27.24 \text{ lbf}$$

Wind Load on Front Face

$F_{S2} := F_S$

$$\frac{F_{S2}}{2} = 16.5 \text{ lbf}$$

Wind Load on Side Face

$DL_{2i} := DL_{ice}$

$$\frac{DL_{2i}}{2} = 23.09 \text{ lbf}$$

Dead Load Ice

$F_{F2i} := F_{Fi}$

$$\frac{F_{F2i}}{2} = 5.55 \text{ lbf}$$

Wind Load with Ice on Front Face

$F_{S2i} := F_{Si}$

$$\frac{F_{S2i}}{2} = 3.74 \text{ lbf}$$

Wind Load with Ice on Side Face

$F_{F2m} := F_{Fm}$

$$\frac{F_{F2m}}{2} = 2.14 \text{ lbf}$$

Wind Load (maintenance) on Front Face

$F_{S2m} := F_{Sm}$

$$\frac{F_{S2m}}{2} = 1.3 \text{ lbf}$$

Wind Load (maintenance) on Side Face

$F_{Eh2} := \text{weight} \cdot A_s \cdot C_s$

$$\frac{F_{Eh2}}{2} = 5 \text{ lbf}$$

Lateral Earthquake Load

$F_{Ev2} := 0.2 \cdot S_{DS} \cdot \text{weight}$

$$\frac{F_{Ev2}}{2} = 0.67 \text{ lbf}$$

Vertical Earthquake Load

New Fujitsu TA08025-B604 Radio

Height

height := 15.75in

Width/Diameter

width := 14.96in

Depth/Diameter

depth := 7.87in

Weight

weight := 63.93lbf

Shape

☒ Flat
☐ Round



DL₃ := weight

$$\frac{DL_3}{2} = 31.96 \text{ lbf}$$

Dead Load

F_{F3} := F_F

$$\frac{F_{F3}}{2} = 27.24 \text{ lbf}$$

Wind Load on Front Face

F_{S3} := F_S

$$\frac{F_{S3}}{2} = 14.33 \text{ lbf}$$

Wind Load on Side Face

DL_{3i} := DL_{ice}

$$\frac{DL_{3i}}{2} = 20.85 \text{ lbf}$$

Dead Load Ice

F_{F3i} := F_{Fi}

$$\frac{F_{F3i}}{2} = 5.55 \text{ lbf}$$

Wind Load with Ice on Front Face

F_{S3i} := F_{Si}

$$\frac{F_{S3i}}{2} = 3.38 \text{ lbf}$$

Wind Load with Ice on Side Face

F_{F3m} := F_{Fm}

$$\frac{F_{F3m}}{2} = 2.14 \text{ lbf}$$

Wind Load (maintenance) on Front Face

F_{S3m} := F_{Sm}

$$\frac{F_{S3m}}{2} = 1.13 \text{ lbf}$$

Wind Load (maintenance) on Side Face

F_{Eh3} := weight·A_S·C_S

$$\frac{F_{Eh3}}{2} = 4.27 \text{ lbf}$$

Lateral Earthquake Load

F_{Ev3} := 0.2·S_{DS}·weight

$$\frac{F_{Ev3}}{2} = 0.57 \text{ lbf}$$

Vertical Earthquake Load

New Raycap RDIDC-4045-PF-48

Height

height := 16.58in

Width/Diameter

width := 14.58in

Depth/Diameter

depth := 8.46in

Weight

weight := 21.85lbf

Shape

☒ Flat
☐ Round



$DL_4 := \text{weight}$

$$\frac{DL_4}{2} = 10.93 \text{ lbf}$$

Dead Load

$F_{F4} := F_F$

$$\frac{F_{F4}}{2} = 27.95 \text{ lbf}$$

Wind Load on Front Face

$F_{S4} := F_S$

$$\frac{F_{S4}}{2} = 16.22 \text{ lbf}$$

Wind Load on Side Face

$DL_{4i} := DL_{ice}$

$$\frac{DL_{4i}}{2} = 22.29 \text{ lbf}$$

Dead Load Ice

$F_{F4i} := F_{Fi}$

$$\frac{F_{F4i}}{2} = 5.67 \text{ lbf}$$

Wind Load with Ice on Front Face

$F_{S4i} := F_{Si}$

$$\frac{F_{S4i}}{2} = 3.72 \text{ lbf}$$

Wind Load with Ice on Side Face

$F_{F4m} := F_{Fm}$

$$\frac{F_{F4m}}{2} = 2.2 \text{ lbf}$$

Wind Load (maintenance) on Front Face

$F_{S4m} := F_{Sm}$

$$\frac{F_{S4m}}{2} = 1.27 \text{ lbf}$$

Wind Load (maintenance) on Side Face

$F_{Eh4} := \text{weight} \cdot A_s \cdot C_s$

$$\frac{F_{Eh4}}{2} = 1.46 \text{ lbf}$$

Lateral Earthquake Load

$F_{Ev4} := 0.2 \cdot S_{DS} \cdot \text{weight}$

$$\frac{F_{Ev4}}{2} = 0.19 \text{ lbf}$$

Vertical Earthquake Load

Antenna Mounting Pipe

$$H_{\text{pipe}} := 12 \cdot \text{ft}$$

$$\text{Pipe} := 2.5$$



$$W_{t_{p1}} := \text{Weight}_{\text{total}}$$

$$W_{t_{p1}} = 69.6 \text{ lbf}$$

Total Weight of Pipe

$$F_{p1} := F_p$$

$$F_{p1} = 95.89 \text{ lbf}$$

Wind Load

$$F_{p1_linear} := F_{p_linear}$$

$$F_{p1_linear} = 8 \cdot \text{plf}$$

Linear Wind Load on Pipe

$$D_{L_{ice,p1}} := D_{L_{ice}}$$

$$D_{L_{ice,p1}} = 104.18 \text{ lbf}$$

Total Weight of Ice

$$D_{L_{ice,p1_linear}} := \frac{D_{L_{ice}}}{H_{\text{pipe}}}$$

$$D_{L_{ice,p1_linear}} = 8.68 \cdot \text{plf}$$

Linear Ice Load on Pipe

$$F_{p1i} := F_{pi}$$

$$F_{p1i} = 28.44 \text{ lbf}$$

Wind Load with Ice

$$F_{p1i_linear} := F_{pi_linear}$$

$$F_{p1i_linear} = 2 \cdot \text{plf}$$

Linear Wind Load with Ice on Pipe

$$F_{p1m} := F_{pm}$$

$$F_{p1m} = 7.54 \text{ lbf}$$

Wind Load (maintenance)

$$F_{p1m_linear} := F_{pm_linear}$$

$$F_{p1m_linear} = 1 \cdot \text{plf}$$

Linear Wind Load (maintenance) on Pipe

$$F_{Ehp1} := \text{Weight}_{\text{total}} \cdot A_s \cdot C_s$$

$$F_{Ehp1} = 9.29 \text{ lbf}$$

Total Lateral Earthquake Load

$$F_{Ehp1_linear} := \frac{F_{Ehp1}}{H_{\text{pipe}}}$$

$$F_{Ehp1_linear} = 0.77 \cdot \text{plf}$$

Linear Lateral Earthquake Load

$$F_{Ev1} := 0.2 \cdot S_{DS} \cdot \text{Weight}_{\text{total}}$$

$$F_{Ev1} = 1.24 \text{ lbf}$$

Total Vertical Earthquake Load

$$F_{Ev1_linear} := \frac{F_{Ev1}}{H_{\text{pipe}}}$$

$$F_{Ev1_linear} = 0.1 \cdot \text{plf}$$

Linear Vertical Earthquake Load

SBWM:

Height

height := 14in

Width/Diameter

width := 11.4in

Depth/Diameter

depth := 2.5in

Weight

weight := 8.13plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 32 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 8 \cdot plf$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1_linear}} := \frac{D_{L_{ice}}}{height}$$

$$D_{L_{ice.M1_linear}} = 15.48 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 7 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 3 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FmM1_linear} := F_{Fm_linear}$$

$$F_{FmM1_linear} = 2 \cdot plf$$

Linear Wind Load (maintenance) on Front Face

$$F_{SmM1_linear} := F_{Sm_linear}$$

$$F_{SmM1_linear} = 1 \cdot plf$$

Linear Wind Load (maintenance) on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 36.9 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 9.02 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 8.09 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 3.21 \cdot lbf$$

Wind Load with Ice on Side Face

$$F_{FmM1} := F_{Fm}$$

$$F_{FmM1} = 2.9 \cdot lbf$$

Wind Load (maintenance) on Front Face

$$F_{SmM1} := F_{Sm}$$

$$F_{SmM1} = 0.74 \cdot lbf$$

Wind Load (maintenance) on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 18.06 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 9.48 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 1.27 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 1.09 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 0.17 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.14 \cdot plf$$

Linear Vertical Earthquake Load

Antenna Mounting Pipe

$$H_{\text{pipe}} := 9 \cdot \text{ft}$$

$$\text{Pipe} := 2.0$$



$$W_{t_{p2}} := \text{Weight}_{\text{total}}$$

$$W_{t_{p2}} = 32.94 \text{ lbf}$$

Total Weight of Pipe

$$F_{p2} := F_p$$

$$F_{p2} = 59.43 \text{ lbf}$$

Wind Load

$$F_{p2_linear} := F_{p_linear}$$

$$F_{p2_linear} = 7 \cdot \text{plf}$$

Linear Wind Load on Pipe

$$D_{L_{ice,p2}} := D_{L_{ice}}$$

$$D_{L_{ice,p2}} = 69.39 \text{ lbf}$$

Total Weight of Ice

$$D_{L_{ice,p2_linear}} := \frac{D_{L_{ice}}}{H_{\text{pipe}}}$$

$$D_{L_{ice,p2_linear}} = 7.71 \cdot \text{plf}$$

Linear Ice Load on Pipe

$$F_{p2i} := F_{pi}$$

$$F_{p2i} = 18.12 \text{ lbf}$$

Wind Load with Ice

$$F_{p2i_linear} := F_{pi_linear}$$

$$F_{p2i_linear} = 2 \cdot \text{plf}$$

Linear Wind Load with Ice on Pipe

$$F_{p2m} := F_{pm}$$

$$F_{p2m} = 4.67 \text{ lbf}$$

Wind Load (maintenance)

$$F_{p2m_linear} := F_{pm_linear}$$

$$F_{p2m_linear} = 1 \cdot \text{plf}$$

Linear Wind Load (maintenance) on Pipe

$$F_{Ehp2} := \text{Weight}_{\text{total}} \cdot A_s \cdot C_s$$

$$F_{Ehp2} = 4.4 \text{ lbf}$$

Total Lateral Earthquake Load

$$F_{Ehp2_linear} := \frac{F_{Ehp2}}{H_{\text{pipe}}}$$

$$F_{Ehp2_linear} = 0.49 \cdot \text{plf}$$

Linear Lateral Earthquake Load

$$F_{Evp2} := 0.2 \cdot S_{DS} \cdot \text{Weight}_{\text{total}}$$

$$F_{Evp2} = 0.59 \text{ lbf}$$

Total Vertical Earthquake Load

$$F_{Evp2_linear} := \frac{F_{Evp2}}{H_{\text{pipe}}}$$

$$F_{Evp2_linear} = 0.07 \cdot \text{plf}$$

Linear Vertical Earthquake Load

Mounting Pipe

$$H_{pipe} := 11.5 \cdot ft$$

$$Pipe := 3.0$$



$$W_{tp2} := Weight_{total}$$

$$W_{tp2} = 87.17 \cdot lbf$$

Total Weight of Pipe

$$F_{p2} := F_p$$

$$F_{p2} = 111.68 \cdot lbf$$

Wind Load

$$F_{p2_linear} := F_{p_linear}$$

$$F_{p2_linear} = 10 \cdot plf$$

Linear Wind Load on Pipe

$$DL_{ice,p2} := DL_{ice}$$

$$DL_{ice,p2} = 113.68 \cdot lbf$$

Total Weight of Ice

$$DL_{ice,p2_linear} := \frac{DL_{ice}}{H_{pipe}}$$

$$DL_{ice,p2_linear} = 9.89 \cdot plf$$

Linear Ice Load on Pipe

$$F_{p2i} := F_{pi}$$

$$F_{p2i} = 28.29 \cdot lbf$$

Wind Load with Ice

$$F_{p2i_linear} := F_{pi_linear}$$

$$F_{p2i_linear} = 2 \cdot plf$$

Linear Wind Load with Ice on Pipe

$$F_{p2m} := F_{pm}$$

$$F_{p2m} = 8.78 \cdot lbf$$

Wind Load (maintenance)

$$F_{p2m_linear} := F_{pm_linear}$$

$$F_{p2m_linear} = 1 \cdot plf$$

Linear Wind Load (maintenance) on Pipe

$$F_{Ehp2} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{Ehp2} = 11.64 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{Ehp2_linear} := \frac{F_{Ehp2}}{H_{pipe}}$$

$$F_{Ehp2_linear} = 1.01 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{Evp2} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

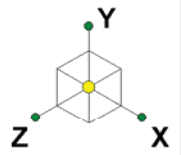
$$F_{Evp2} = 1.55 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{Evp2_linear} := \frac{F_{Evp2}}{H_{pipe}}$$

$$F_{Evp2_linear} = 0.13 \cdot plf$$

Linear Vertical Earthquake Load



Proposed Antenna
Typ. of 1 per sector

Proposed RRH's
Typ. of 2 per sector

Proposed Raycap
Typ. of 1 per sector

3'-6"

New Slider Bracket
Wall Mount
MFR: Site Pro 1
Part# SBWM
Typ. of 1

New Pipe 2½ Std. x 12'-0"
Typ. of 1 at sector Alpha

Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116B

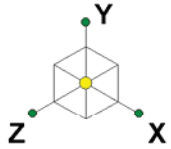
Mount Analysis (Sector Alpha)

Rendered

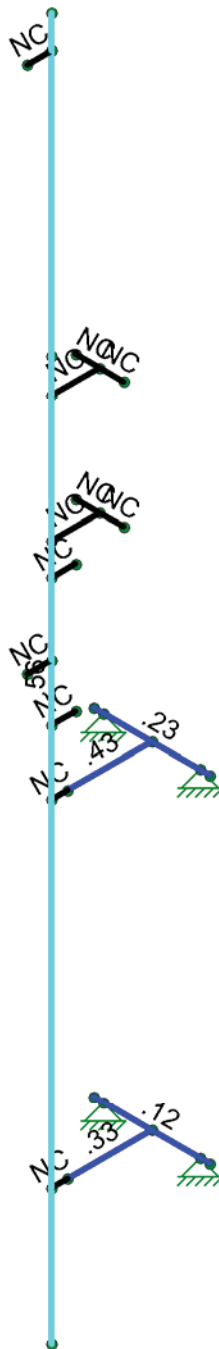
SK - 1

June 2, 2022 at 8:37 AM

MWMKE00120 - Mount Analysis (S...

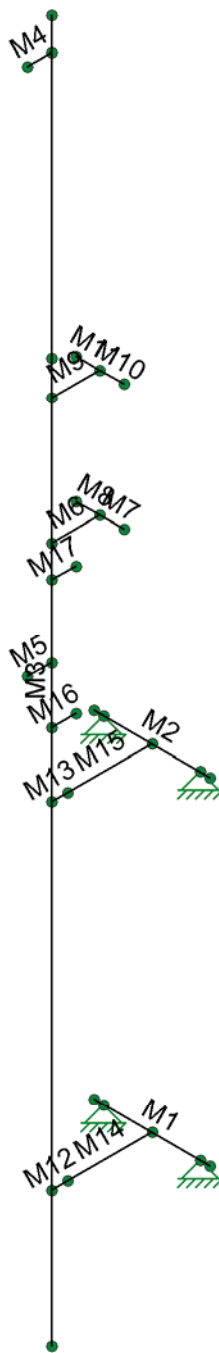
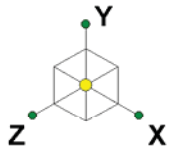


Code Check (Env)	
	No Calc
	> 1.0
	.90-1.0
	.75-.90
	.50-.75
	0-.50



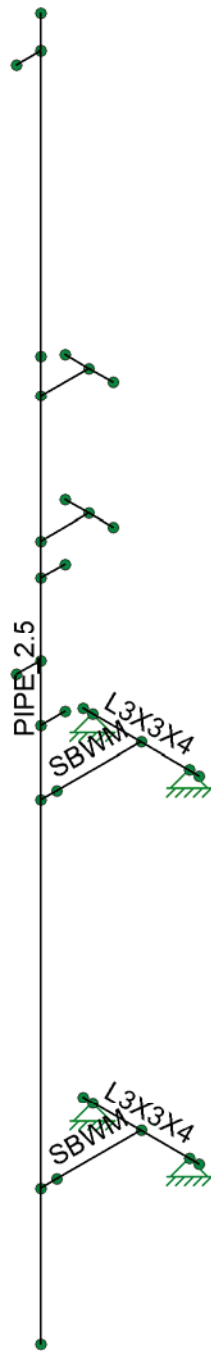
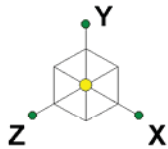
Member Code Checks Displayed (Enveloped)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Alpha) Unity Bending	SK - 2
AA		June 2, 2022 at 8:37 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



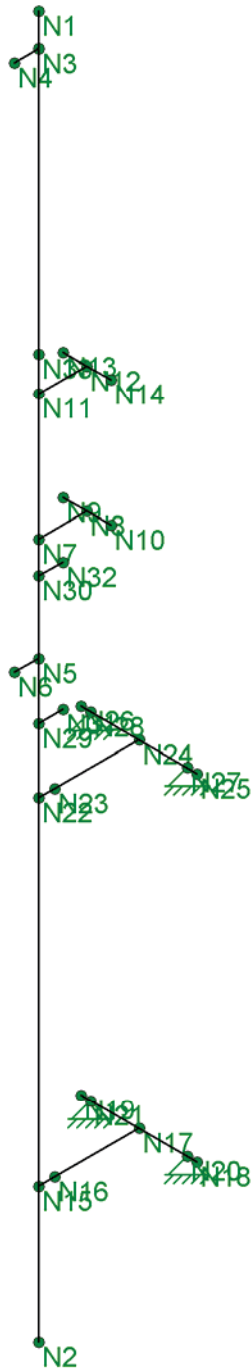
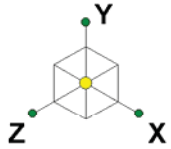
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Alpha) Members	SK - 3
AA		June 2, 2022 at 8:38 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



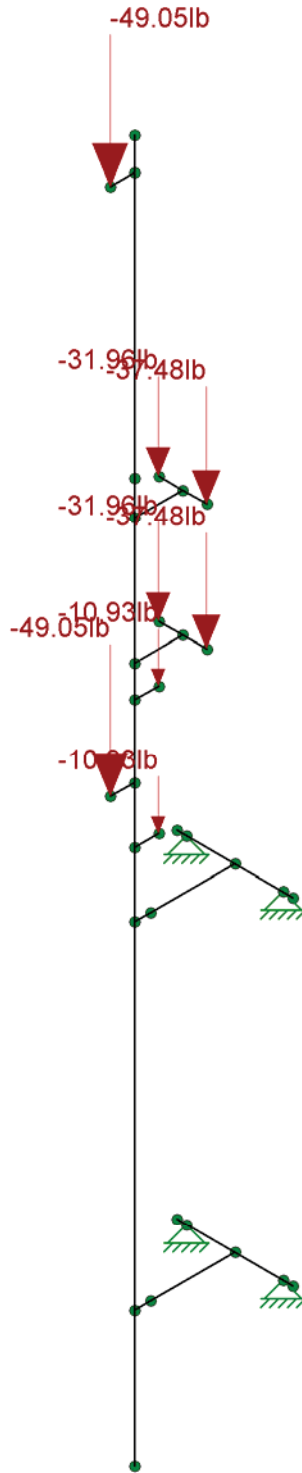
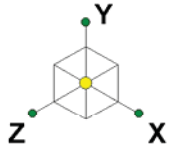
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Alpha) Shapes	SK - 4
AA		June 2, 2022 at 8:38 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



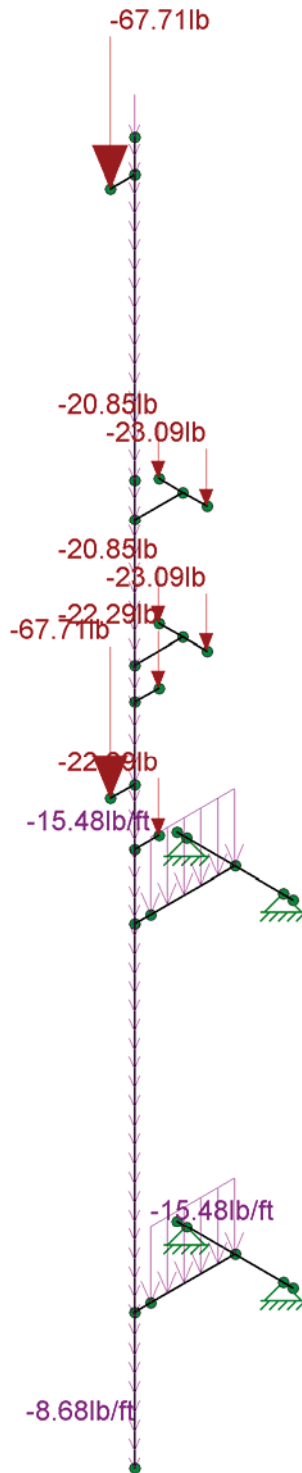
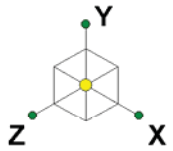
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Alpha) Nodes	SK - 5
AA		June 2, 2022 at 8:38 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 1, DL
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Alpha) Dead Load	SK - 6
AA		June 2, 2022 at 8:39 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...

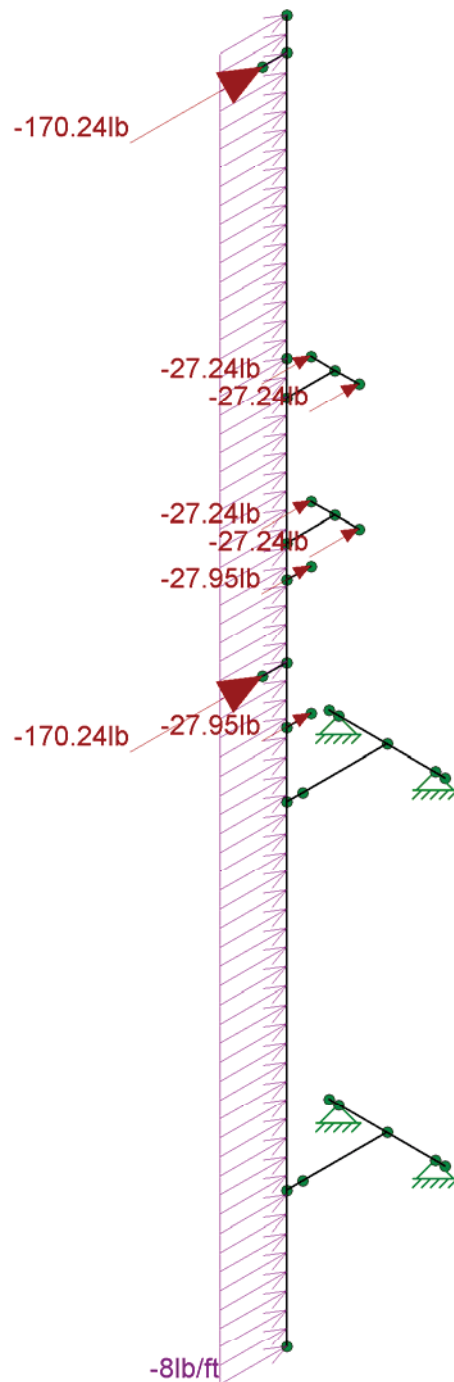
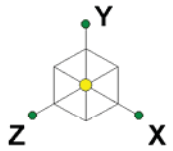


Loads: BLC 2, DLI
Envelope Only Solution

Fullerton Engineering Cons...
AA
MWMKE00116B

Mount Analysis (Sector Alpha)
Dead Load with Ice

SK - 7
June 2, 2022 at 8:39 AM
MWMKE00120 - Mount Analysis (S...



Loads: BLC 3, WL(0)
Envelope Only Solution

Fullerton Engineering Cons...

AA

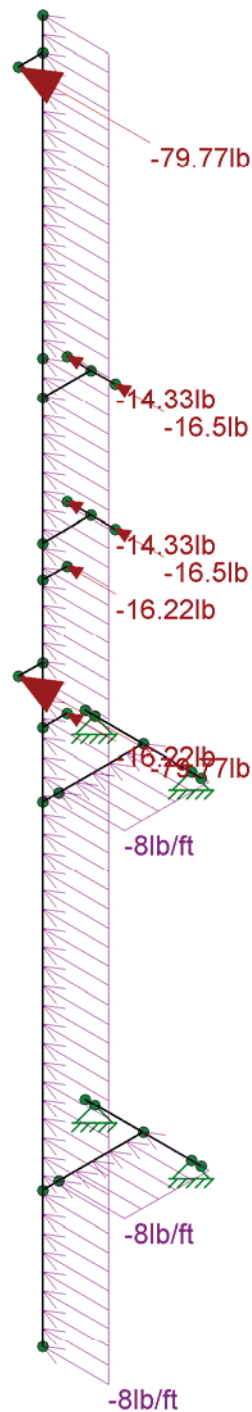
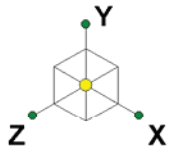
MWMKE00116B

Mount Analysis (Sector Alpha)
Wind Load Z

SK - 8

June 2, 2022 at 8:39 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 4, WL(90)
Envelope Only Solution

Fullerton Engineering Cons...

AA

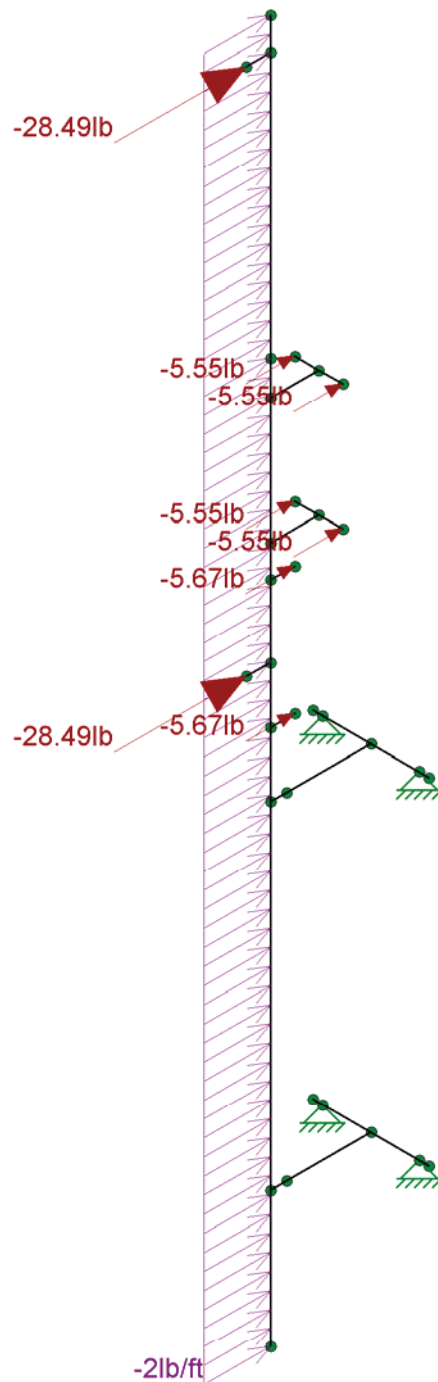
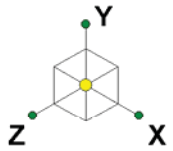
MWMKE00116B

Mount Analysis (Sector Alpha)
Wind Load X

SK - 9

June 2, 2022 at 8:40 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 5, WL.i(0)
Envelope Only Solution

Fullerton Engineering Cons...

AA

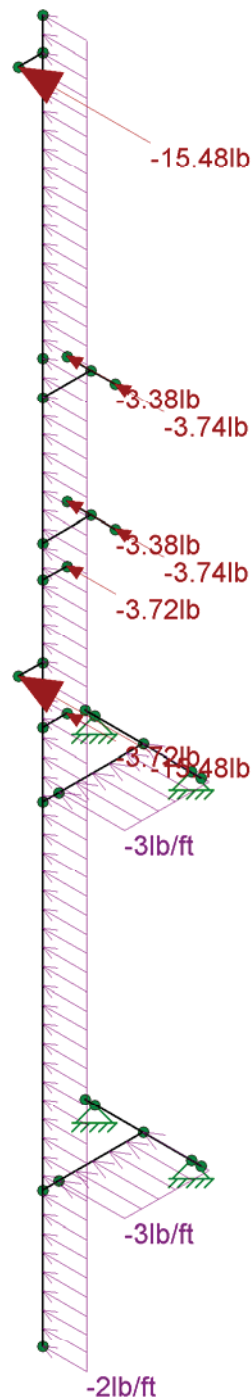
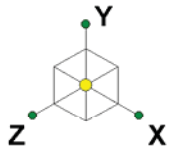
MWMKE00116B

Mount Analysis (Sector Alpha)
Wind Load with Ice Z

SK - 10

June 2, 2022 at 8:40 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 6, WL.i(90)
Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116B

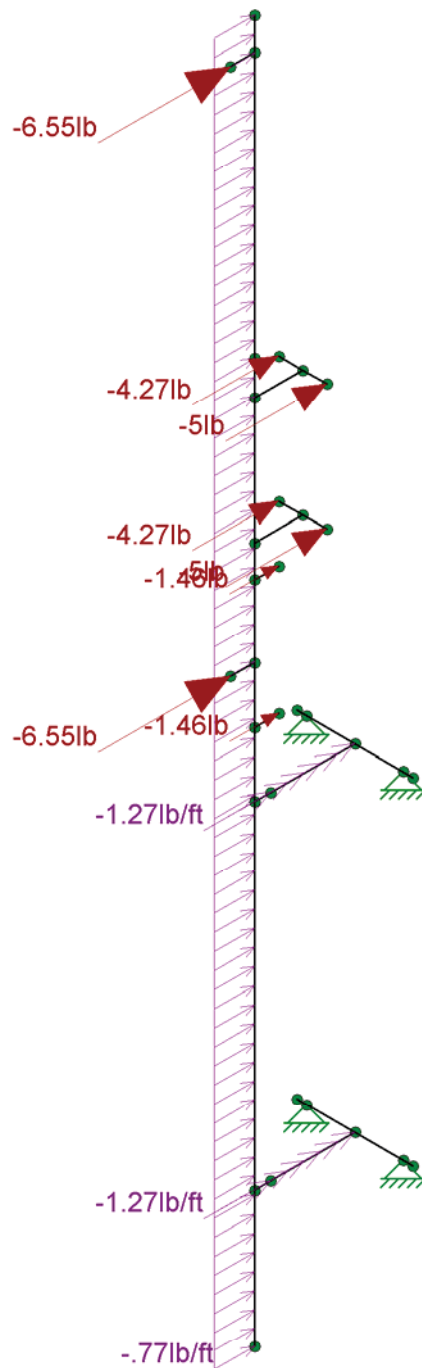
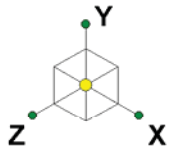
Mount Analysis (Sector Alpha)

Wind Load with Ice X

SK - 11

June 2, 2022 at 8:41 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 8, EH(0)
Envelope Only Solution

Fullerton Engineering Cons...

AA

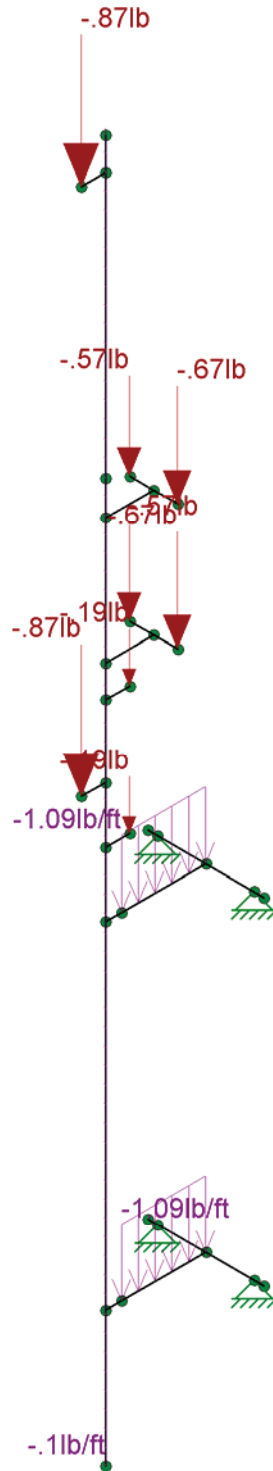
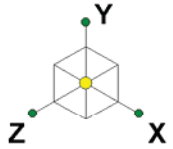
MWMKE00116B

Mount Analysis (Sector Alpha)
Seismic Load (Lateral)

SK - 12

June 2, 2022 at 8:41 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 10, EV
Envelope Only Solution

Fullerton Engineering Cons...
AA
MWMKE00116B

Mount Analysis (Sector Alpha)
Seismic Load (Vertical)

SK - 13
June 2, 2022 at 8:42 AM
MWMKE00120 - Mount Analysis (S...



Company : Fullerton Engineering Consultants, LLC
 Designer : AA
 Job Number : MWMKE00116B
 Model Name : Mount Analysis (Sector Alpha)

June 2, 2022
 8:43 AM
 Checked By: BK

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver
Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

(Global) Model Settings, Continued

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

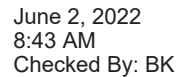
Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torq...	Kyy	Kzz	Cb	Function
1	M1	L3X3X4	14.5	12	12	12	12	12	2.1	2.1		Lateral
2	M2	L3X3X4	14.5	12	12	12	12	12	2.1	2.1		Lateral
3	M3	PIPE 2.5	144	48	48	48	48	48	2.1	2.1		Lateral
4	M14	SBWM	10.5			Lbyy			.65	.65		Lateral
5	M15	SBWM	10.5			Lbyy			.65	.65		Lateral

Material Takeoff

	Material	Size	Pieces	Length[in]	Weight[LB]
1	General				
2	RIGID		12	40	0
3	Total General		12	40	0
4					
5	Hot Rolled Steel				
6	A36 Gr.36	L3X3X4	2	29	11.842
7	A36 Gr.36	SBWM	2	21	17.936

Page 34 of 108

Load Combinations (Continued)

	Description	So...	P...	S...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...
35	0.9*DL+1.0*EH(0)...	Yes	Y		1	.9	8	1	10	1						
36	0.9*DL+1.0*EH(90)...	Yes	Y		1	.9	9	1	10	-1						
37	0.9*DL+1.0*EH(90)...	Yes	Y		1	.9	9	1	10	1						
38	0.9*DL+1.0*EH(18...	Yes	Y		1	.9	8	-1	10	-1						
39	0.9*DL+1.0*EH(18...	Yes	Y		1	.9	8	-1	10	1						
40	0.9*DL+1.0*EH(27...	Yes	Y		1	.9	9	-1	10	-1						
41	0.9*DL+1.0*EH(27...	Yes	Y		1	.9	9	-1	10	1						
42	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	1						
43	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.866	12	.5				
44	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.5	12	.866				
45	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	12	1						
46	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.5	12	.866				
47	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.866	12	.5				
48	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.1						
49	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.866	12	-.5				
50	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.5	12	-.866				
51	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	12	-.1						
52	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.5	12	-.866				
53	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.866	12	-.5				

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-in]	LC	MY [lb-in]	LC	MZ [lb-in]	LC
1	N28	max	234.481	5	930.532	4	681.405	3	0	53	0	53	0	53
2		min	-234.961	11	-696.816	10	-783.269	9	0	1	0	1	0	1
3	N27	max	234.481	5	934.716	12	682.422	13	0	53	0	53	0	53
4		min	-234.961	11	-692.632	6	-782.25	7	0	1	0	1	0	1
5	N21	max	53.144	11	507.455	9	336.424	9	0	53	0	53	0	53
6		min	-52.664	5	-320.423	3	-234.439	3	0	1	0	1	0	1
7	N20	max	53.144	11	506.583	7	335.406	7	0	53	0	53	0	53
8		min	-52.664	5	-321.295	13	-235.457	13	0	1	0	1	0	1
9	Totals:	max	363.633	5	824.362	14	601.301	2						
10		min	-363.633	11	311.216	38	-601.301	8						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

Member	Shape	Code Check	Loc[in]	LC	Shear Ch...	Loc[in]	Dir	LC	phi*Pnc	phi*Pnt	phi*Mn	phi*Mn	Cb	Eqn
1	M3	PIPE 2.5	.561	60	2	.045	58.5	8	28468.4...	50715	43155	43155	1	H1-1b
2	M15	SBWM	.432	0	13	.150	4.266	z 12	82203.8...	97588.8	31644.4...	356724	1.526	H1-1b
3	M14	SBWM	.334	0	8	.060	4.922	z 9	82203.8...	97588.8	31644.4...	356724	1.495	H1-1b
4	M2	L3X3X4	.235	7.25	10	.064	1.359	y 12	42313.8...	46656	20257.6...	45068.9...	1	H2-1
5	M1	L3X3X4	.115	7.25	9	.035	13.1...	y 9	42313.8...	46656	20257.6...	45068.9...	1	H2-1

Max. Stress Ratio < 1
 Members are adequate

Wall Mount Connection Calculations (Sector Alpha)

New wall mounts will be attached to existing elevated roof wall via Thru-Bolts ½" Ø

Maximum Reactions from Risa Mount Analysis per one thru-bolt:

3D Envelope Joint Reactions															
	Joint		X [lb]	L...	Y [lb]	L...	Z [lb]	L...	MX [lb-i...	L...	MY [lb-i...	L...	MZ [lb-i...	L...	
1	N28	max	234.481	5	930.532	4	681.405	3	0	53	0	53	0	53	
2		min	-234.961	11	-696.816	10	-783.269	9	0	1	0	1	0	1	
3	N27	max	234.481	5	934.716	12	682.422	13	0	53	0	53	0	53	
4		min	-234.961	11	-692.632	6	-782.25	7	0	1	0	1	0	1	
5	N20	max	53.144	11	506.583	7	335.406	7	0	53	0	53	0	53	
6		min	-52.664	5	-321.295	13	-235.457	13	0	1	0	1	0	1	
7	N21	max	53.144	11	507.455	9	336.424	9	0	53	0	53	0	53	
8		min	-52.664	5	-320.423	3	-234.439	3	0	1	0	1	0	1	
9	Totals:	max	363.633	5	824.362	14	601.301	2							
10		min	-363.633	11	311.216	38	-601.301	8							

$$X := 234.961 \text{ lbf}$$

Maximum Factored Reaction - X direction

$$Y := 930.532 \text{ lbf}$$

Maximum Factored Reaction - Y direction

$$Z := 783.269 \text{ lbf}$$

Maximum Factored Reaction - Z direction

$$P_t := Z$$

$$P_t = 783.27 \text{ lbf}$$

Factored Tensile Force

$$P_v := \sqrt{X^2 + Y^2}$$

$$P_v = 959.74 \text{ lbf}$$

Factored Shear Force

$$d_b := 0.5 \text{ in}$$

Diameter of thru-bolt

$$A_b := 0.25\pi \cdot d_b^2$$

$$A_b = 0.2 \text{ in}^2$$

Area of thru-bolt

$$P_{t_bolt} := P_t$$

$$P_{t_bolt} = 783.27 \text{ lbf}$$

Tension at each thru-bolt

$$P_{v_bolt} := P_v$$

$$P_{v_bolt} = 959.74 \text{ lbf}$$

Shear at each thru-bolt

Tensile and Shear Strength of Thru-Bolts

$$F_{nt} := 45 \text{ksi}$$

Nominal tensile strength per AISC
 360, Table J3.2

$$F_{nv} := 27 \text{ksi}$$

Nominal shear strength per AISC
 360, Table J3.2

$$\phi_{bolt} := 0.75$$

Resistance Factor (LRFD - AISC
 360, Section J3-6)

$$R_{nt} := \phi_{bolt} \cdot F_{nt} \cdot A_b$$

$$R_{nt} = 6.63 \cdot \text{kip}$$

Design Nominal Tensile Strength
 (AISC 360, Section J3-1)

$$R_{nv} := \phi_{bolt} \cdot F_{nv} \cdot A_b$$

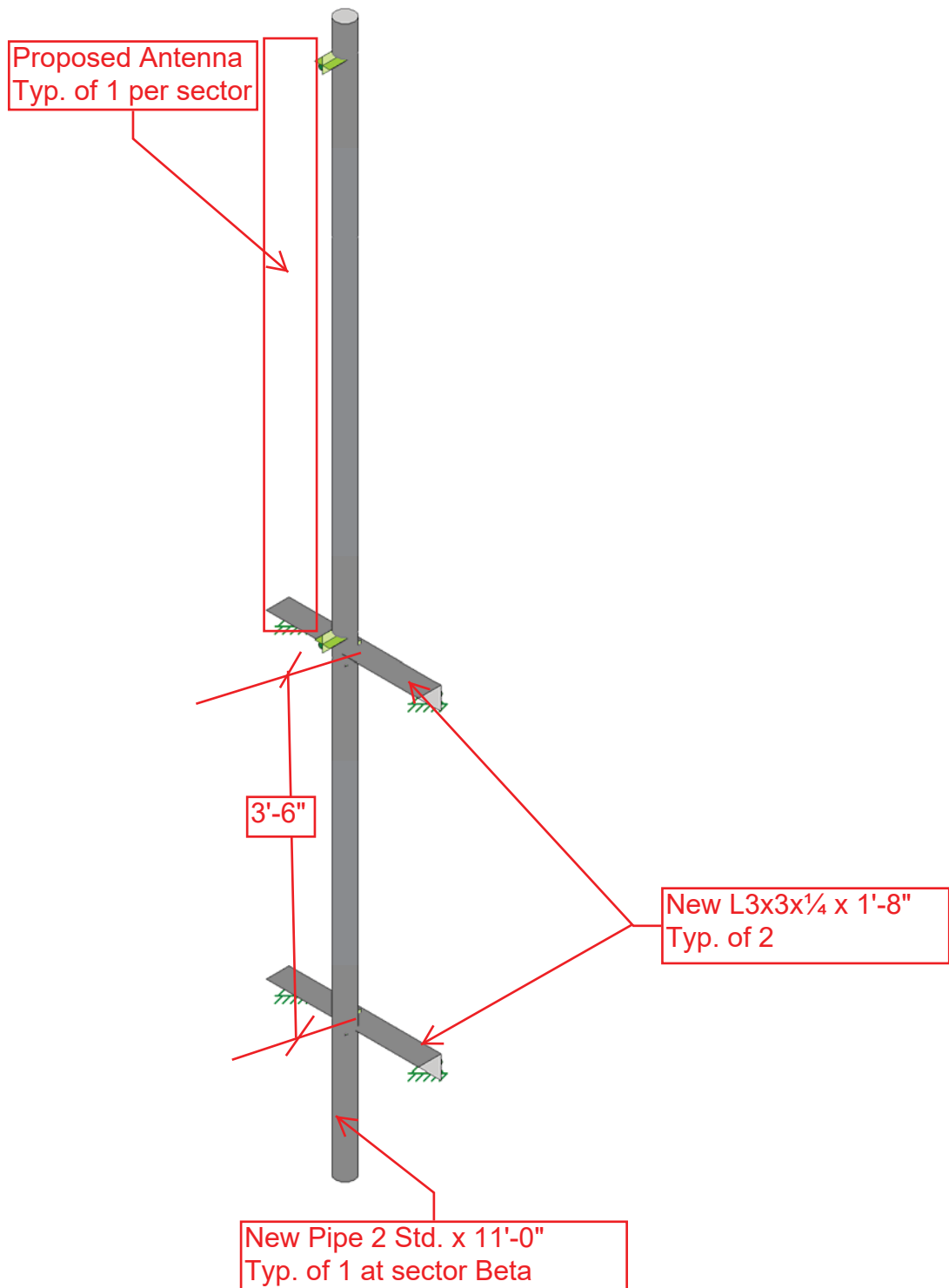
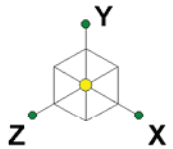
$$R_{nv} = 3.98 \cdot \text{kip}$$

Design Nominal Shear Strength
 (AISC 360, Section J3-1)

$$\frac{P_{t_bolt}}{R_{nt}} = 11.82\% \quad < 30\%$$

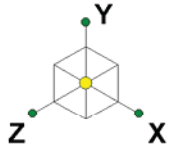
$$\frac{P_{v_bolt}}{R_{nv}} = 24.14\% \quad < 30\%$$

Check = "Thru-Bolts are adequate. Effects of combined stress don't need to be investigated because ratio of either tension or shear is under 30%"

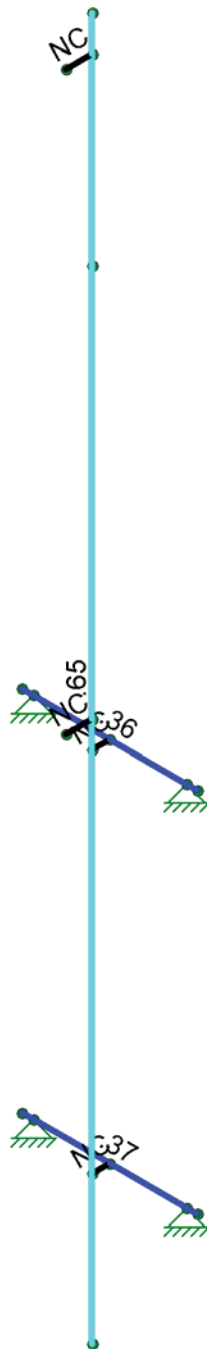


Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Rendered	SK - 1
AA		June 2, 2022 at 10:23 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...

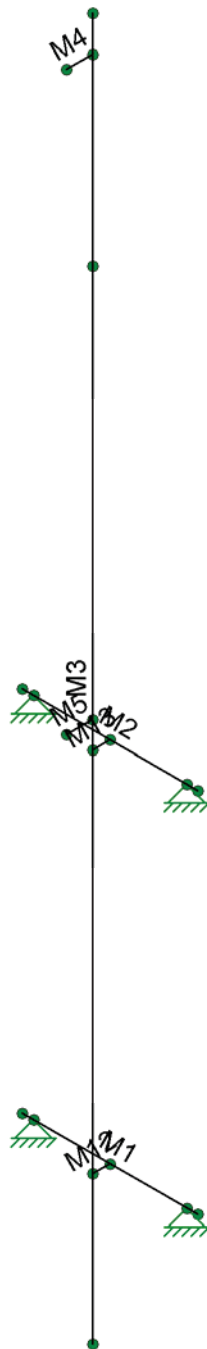
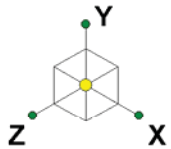


Code Check (Env)	
	No Calc
	> 1.0
	.90-1.0
	.75-.90
	.50-.75
	0-.50



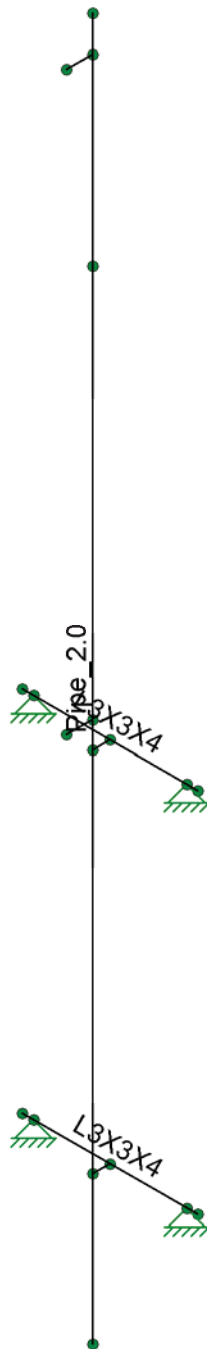
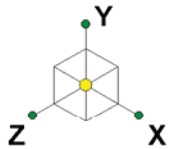
Member Code Checks Displayed (Enveloped)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Unity Bending	SK - 2
AA		June 2, 2022 at 10:23 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



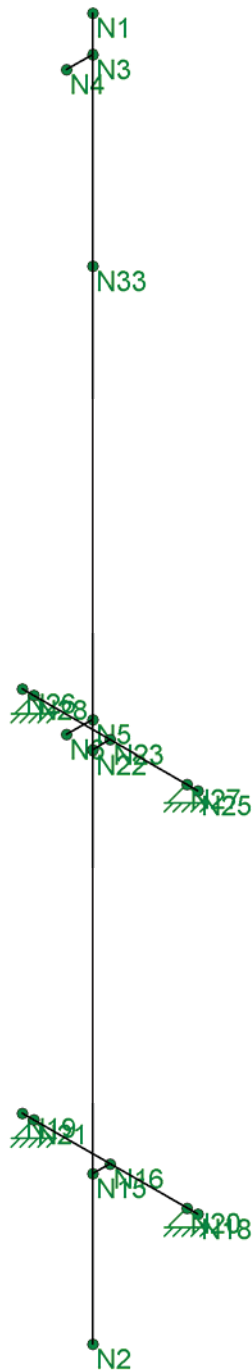
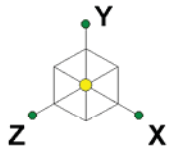
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Members	SK - 3
AA		June 2, 2022 at 10:23 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



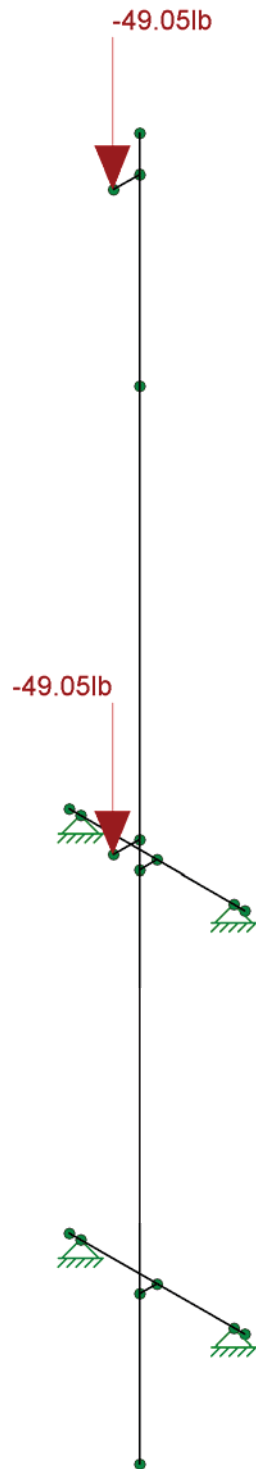
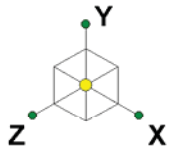
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Shapes	SK - 4
AA		June 2, 2022 at 10:24 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



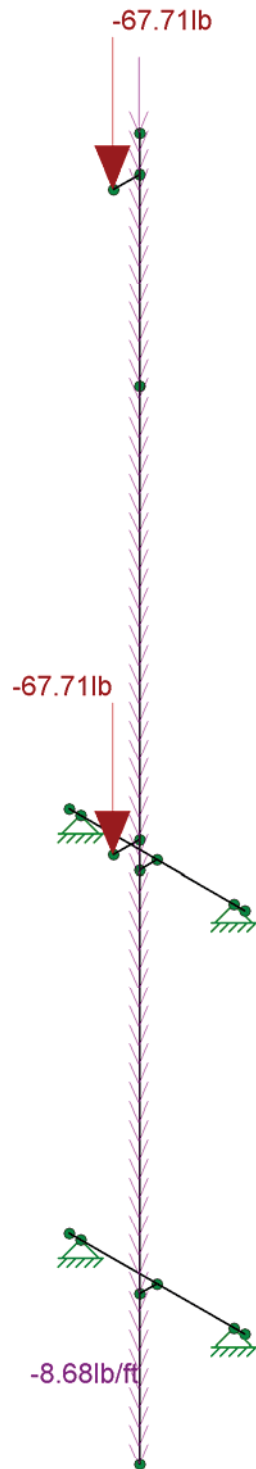
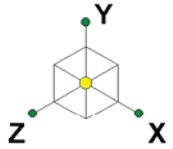
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Shapes	SK - 5
AA		June 2, 2022 at 10:25 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



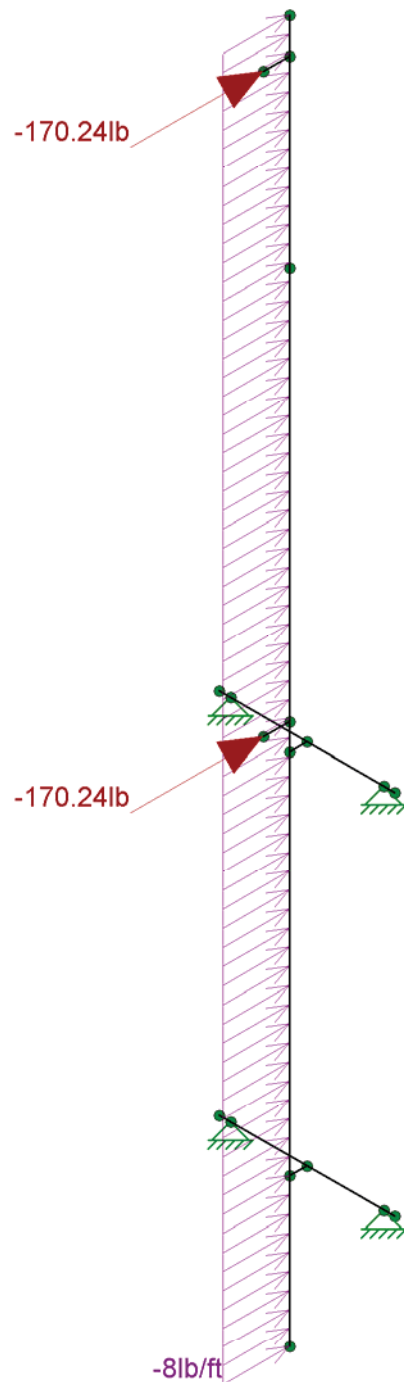
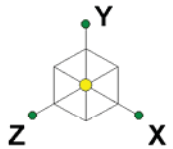
Loads: BLC 1, DL
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Dead Load	SK - 6
AA		June 2, 2022 at 10:25 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 2, DLi
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Dead Load with Ice	SK - 7
AA		June 2, 2022 at 10:26 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 3, WL(0)
Envelope Only Solution

Fullerton Engineering Cons...

AA

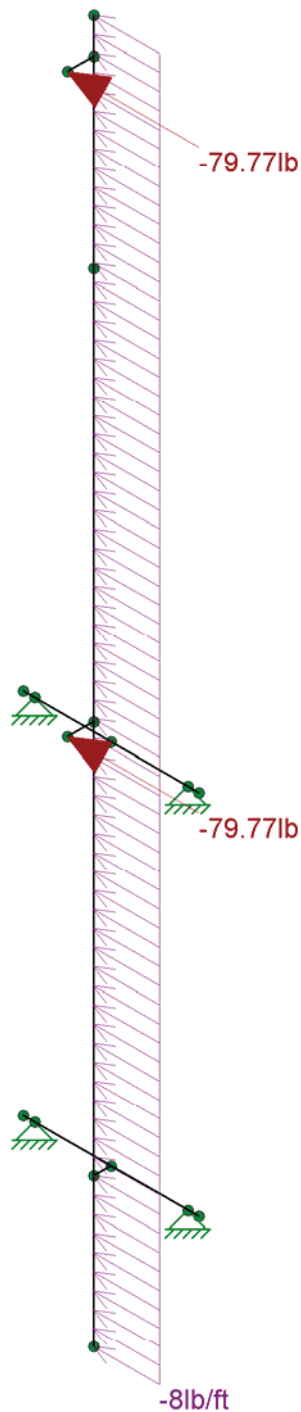
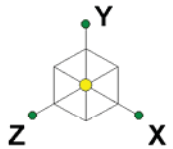
MWMKE00116B

Mount Analysis (Sector Beta)
Wind Load Z

SK - 8

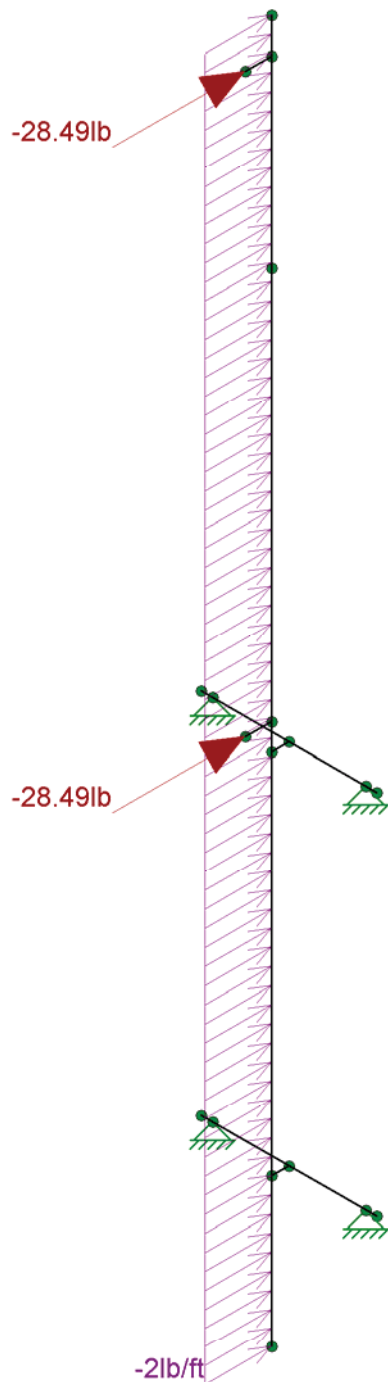
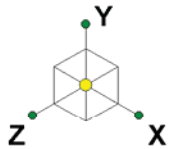
June 2, 2022 at 10:26 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 4, WL(90)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Wind Load X	SK - 9
AA		June 2, 2022 at 10:27 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 5, WL.i(0)
Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116B

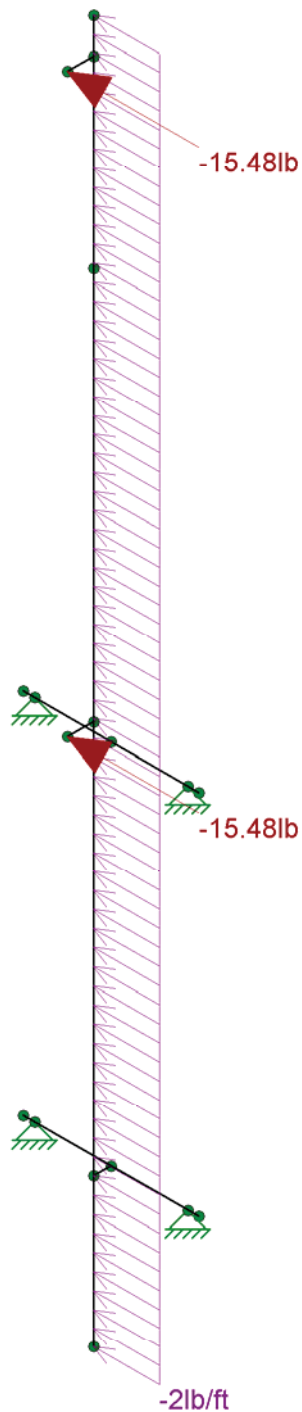
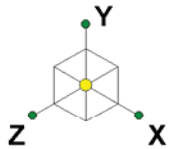
Mount Analysis (Sector Beta)

Wind Load with Ice Z

SK - 10

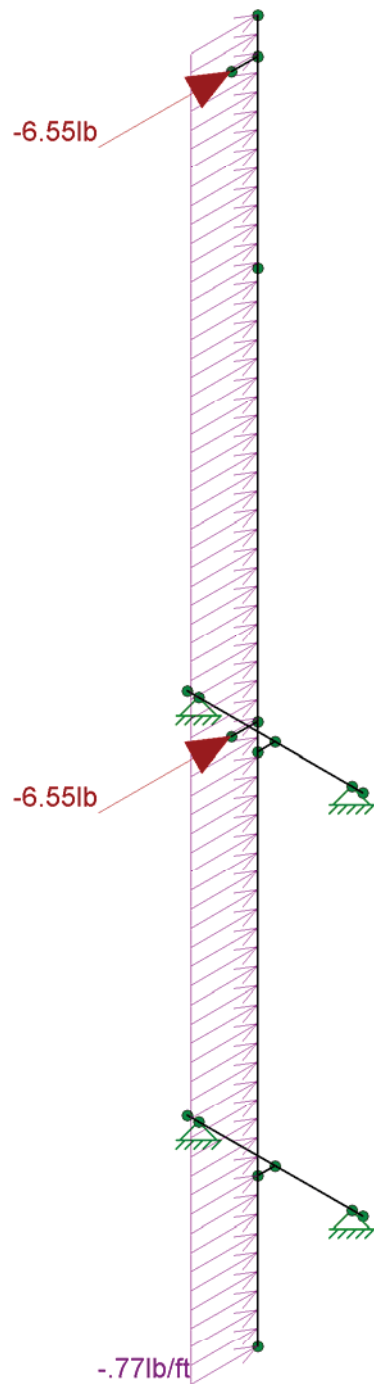
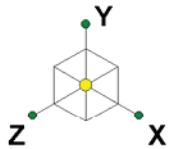
June 2, 2022 at 10:27 AM

MWMKE00120 - Mount Analysis (S...



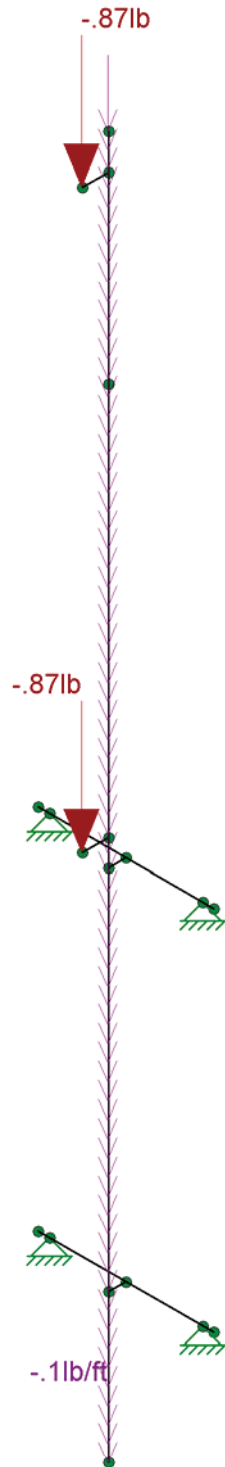
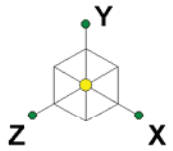
Loads: BLC 6, WL.i(90)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Wind Load with Ice X	SK - 11
AA		June 2, 2022 at 10:28 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 8, EH(0)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Seismic Load (Lateral)	SK - 12
AA		June 2, 2022 at 10:28 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 10, EV
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Beta) Seismic Load (Vertical)	SK - 13
AA		June 2, 2022 at 10:29 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Company : Fullerton Engineering Consultants, LLC
 Designer : AA
 Job Number : MWMKE00116B
 Model Name : Mount Analysis (Sector Beta)

June 2, 2022
 10:30 AM
 Checked By: BK

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver
Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

(Global) Model Settings, Continued

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

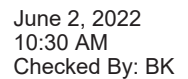
Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torq...	Kyy	Kzz	Cb	Function
1	M1	L3X3X4	20	12	12	12	12	12	2.1	2.1		Lateral
2	M2	L3X3X4	20	12	12	12	12	12	2.1	2.1		Lateral
3	M3	Pipe_2.0	132	48	48	48	48	48	2.1	2.1		Lateral

Material Takeoff

	Material	Size	Pieces	Length[in]	Weight[LB]
1	General				
2	RIGID		4	10	0
3	Total General		4	10	0
4					
5	Hot Rolled Steel				
6	A36 Gr.36	L3X3X4	2	40	16.333
7	A53 Gr. B	Pipe 2.0	1	132	38.179
8	Total HR Steel		3	172	54.513

Page 53 of 108

Load Combinations (Continued)

	Description	So...	P...	S...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...
40	0.9*DL+1.0*EH(27...	Yes	Y		1	.9	9	-1	10	-1									
41	0.9*DL+1.0*EH(27...	Yes	Y		1	.9	9	-1	10	1									
42	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	1									
43	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.866	12	.5							
44	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.5	12	.866							
45	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	12	1									
46	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.5	12	.866							
47	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.866	12	.5							
48	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-1									
49	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.866	12	-.5							
50	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.5	12	-.866							
51	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	12	-1									
52	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.5	12	-.866							
53	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.866	12	-.5							

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-in]	LC	MY [lb-in]	LC	MZ [lb-in]	LC
1	N20	max	.437	17	893.01	8	162.534	8	0	53	0	53	0	53
2		min	-.437	23	-767.5	2	-147.418	2	0	1	0	1	0	1
3	N21	max	.437	17	893.01	8	162.534	8	0	53	0	53	0	53
4		min	-.437	23	-767.5	2	-147.418	2	0	1	0	1	0	1
5	N27	max	124.15	5	942.076	13	361.653	2	0	53	0	53	0	53
6		min	-124.15	11	-885.639	7	-376.768	8	0	1	0	1	0	1
7	N28	max	124.15	5	942.076	3	361.653	2	0	53	0	53	0	53
8		min	-124.15	11	-885.639	9	-376.768	8	0	1	0	1	0	1
9	Totals:	max	247.535	5	414.035	14	428.47	2						
10		min	-247.535	11	134.511	38	-428.469	8						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

	Member	Shape	Code Check	Loc[in]	LC	Shear Ch..	Loc[in]	Dir	LC	phi*Pnc ...	phi*Pnt [...]	phi*Mn ...	phi*Mn ...	Cb	Eqn
1	M3	Pipe 2.0	.649	59.1...	8	.049	59.1...		13	13787.8...	32130	22459.5	22459.5	1	H1-1b
2	M1	L3X3X4	.367	10	8	.061	1.25	y	8	42313.8...	46656	20257.6...	45068.9...	1	H2-1
3	M2	L3X3X4	.360	10	13	.065	1.25	y	13	42313.8...	46656	20257.6...	45068.9...	1	H2-1

Max. Stress Ratio<1
 Members are adequate

Wall Mount Connection Calculations (Sector Beta)

New wall mounts will be attached to existing stairwell wall via Thru-Bolts 1/2" Ø

Maximum Reactions from Risa Mount Analysis per one thru-bolt:

3D Envelope Joint Reactions														
	Joint		X [lb]	L...	Y [lb]	L...	Z [lb]	L...	MX [lb-i...	L...	MY [lb-i...	L...	MZ [lb-i...	L...
1	N27	max	124.15	5	942.076	13	361.653	2	0	53	0	53	0	53
2		min	-124.15	11	-885.639	7	-376.768	8	0	1	0	1	0	1
3	N28	max	124.15	5	942.076	3	361.653	2	0	53	0	53	0	53
4		min	-124.15	11	-885.639	9	-376.768	8	0	1	0	1	0	1
5	N20	max	.437	17	893.01	8	162.534	8	0	53	0	53	0	53
6		min	-.437	23	-767.5	2	-147.418	2	0	1	0	1	0	1
7	N21	max	.437	17	893.01	8	162.534	8	0	53	0	53	0	53
8		min	-.437	23	-767.5	2	-147.418	2	0	1	0	1	0	1
9	Totals:	max	247.535	5	414.035	14	428.47	2						
10		min	-247.535	11	134.511	38	-428.469	8						

$$X := 124.15 \text{ lbf}$$

Maximum Factored Reaction - X direction

$$Y := 942.076 \text{ lbf}$$

Maximum Factored Reaction - Y direction

$$Z := 376.768 \text{ lbf}$$

Maximum Factored Reaction - Z direction

$$P_t := Z$$

$$P_t = 376.77 \text{ lbf}$$

Factored Tensile Force

$$P_v := \sqrt{X^2 + Y^2}$$

$$P_v = 950.22 \text{ lbf}$$

Factored Shear Force

$$d_b := 0.5 \text{ in}$$

Diameter of thru-bolt

$$A_b := 0.25\pi \cdot d_b^2$$

$$A_b = 0.2 \cdot \text{in}^2$$

Area of thru-bolt

$$P_{t_bolt} := P_t$$

$$P_{t_bolt} = 376.77 \text{ lbf}$$

Tension at each thru-bolt

$$P_{v_bolt} := P_v$$

$$P_{v_bolt} = 950.22 \text{ lbf}$$

Shear at each thru-bolt

Tensile and Shear Strength of Thru-Bolts

$$F_{nt} := 45 \text{ ksi}$$

Nominal tensile strength per AISC
 360, Table J3.2

$$F_{nv} := 27 \text{ ksi}$$

Nominal shear strength per AISC
 360, Table J3.2

$$\phi_{bolt} := 0.75$$

Resistance Factor (LRFD - AISC
 360, Section J3-6)

$$R_{nt} := \phi_{bolt} \cdot F_{nt} \cdot A_b$$

$$R_{nt} = 6.63 \cdot \text{kip}$$

Design Nominal Tensile Strength
 (AISC 360, Section J3-1)

$$R_{nv} := \phi_{bolt} \cdot F_{nv} \cdot A_b$$

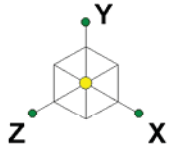
$$R_{nv} = 3.98 \cdot \text{kip}$$

Design Nominal Shear Strength
 (AISC 360, Section J3-1)

$$\frac{P_{t_bolt}}{R_{nt}} = 5.69\% \quad < 30\%$$

$$\frac{P_{v_bolt}}{R_{nv}} = 23.9\% \quad < 30\%$$

Check = "Thru-Bolts are adequate. Effects of combined stress don't need to be investigated because ratio of either tension or shear is under 30%"



Proposed Antenna
Typ. of 1 per sector

Proposed RRH's
Typ. of 2 per sector

Proposed Raycap
Typ. of 1 per sector

New Slider Bracket
Wall Mount
MFR: Site Pro 1
Part# SBWM
Typ. of 1

3'-6"

New Pipe 2½ Std. x 12'-0"
Typ. of 1 at sector Gamma

Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116B

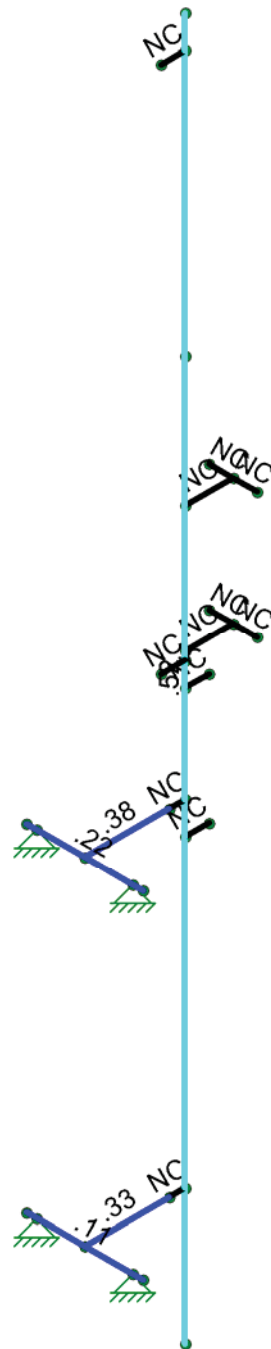
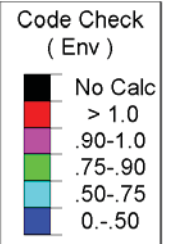
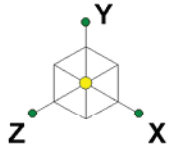
Mount Analysis (Sector Gamma)

Rendered

SK - 1

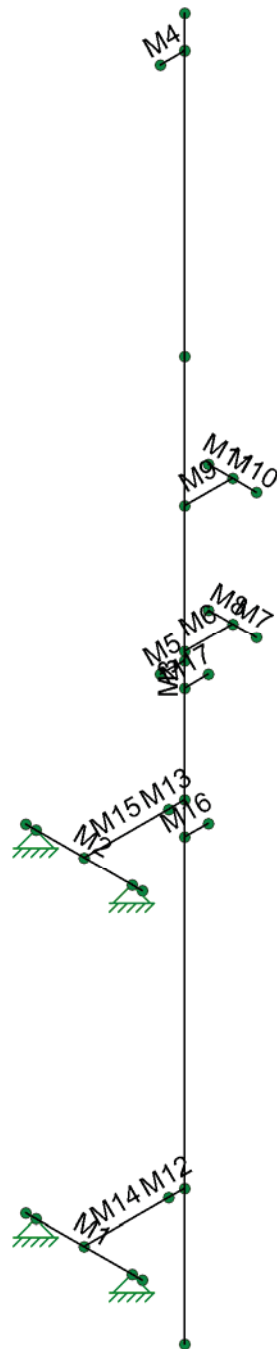
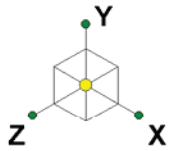
June 2, 2022 at 9:21 AM

MWMKE00120 - Mount Analysis (S...



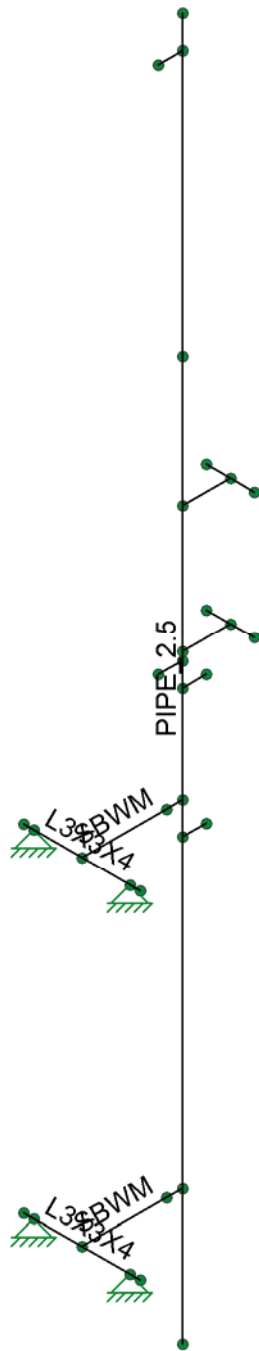
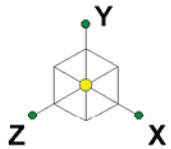
Member Code Checks Displayed (Enveloped)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Unity Bending	SK - 2
AA		June 2, 2022 at 9:21 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



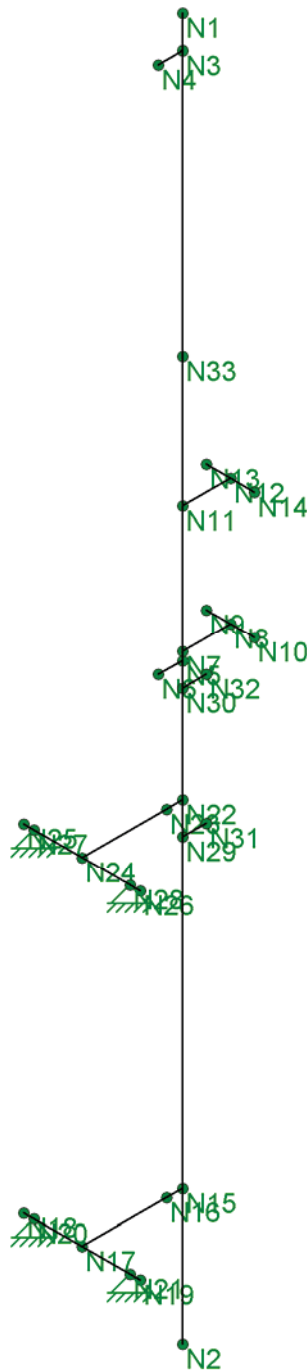
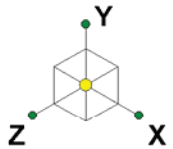
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Members	SK - 3
AA		June 2, 2022 at 9:22 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Envelope Only Solution

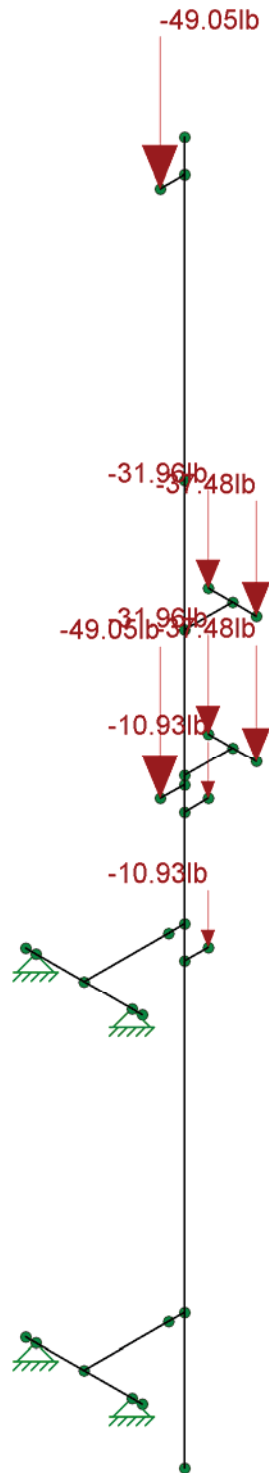
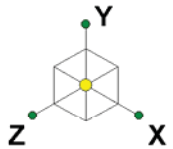
Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Shapes	SK - 4
AA		June 2, 2022 at 9:23 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Envelope Only Solution

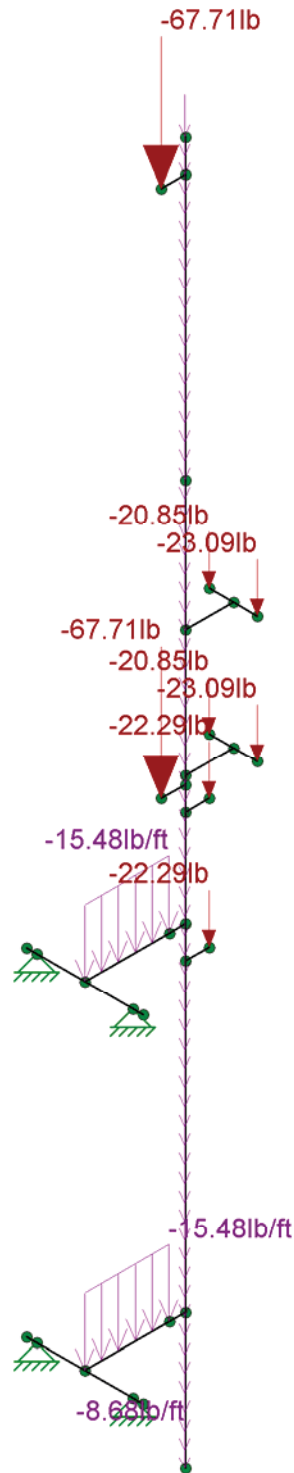
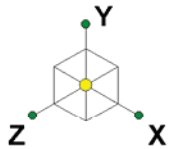
Fullerton Engineering Cons...	Mount Analysis (Sector Gamma)	SK - 5
AA		June 2, 2022 at 9:59 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...

Nodes



Loads: BLC 1, DL
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Dead Load	SK - 6
AA		June 2, 2022 at 10:00 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...

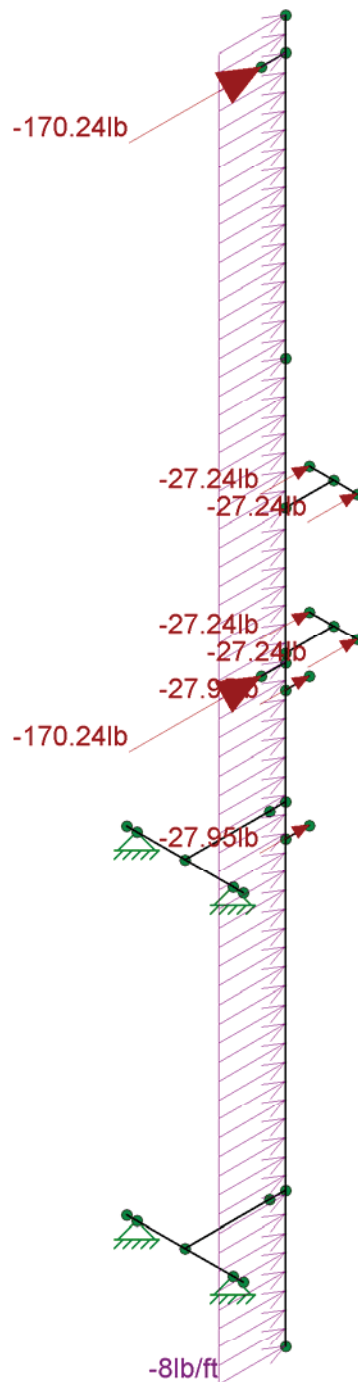
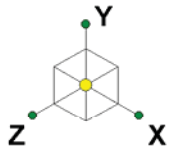


Loads: BLC 2, DLI
Envelope Only Solution

Fullerton Engineering Cons...
AA
MWMKE00116B

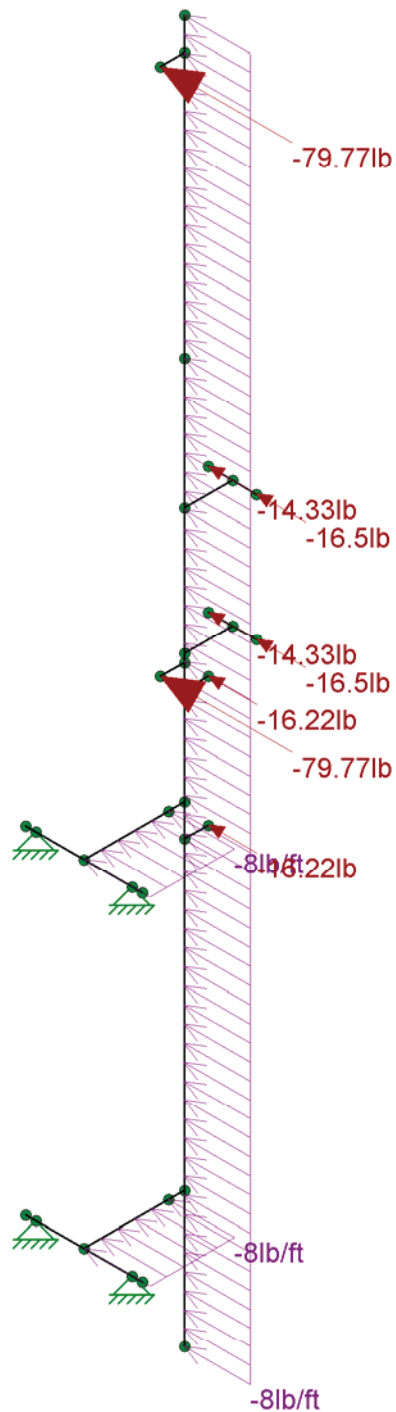
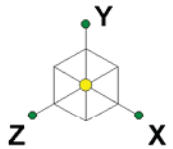
Mount Analysis (Sector Gamma)
Dead Load with Ice

SK - 7
June 2, 2022 at 10:00 AM
MWMKE00120 - Mount Analysis (S...



Loads: BLC 3, WL(0)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Wind Load Z	SK - 8
AA		June 2, 2022 at 10:00 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 4, WL(90)
Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116B

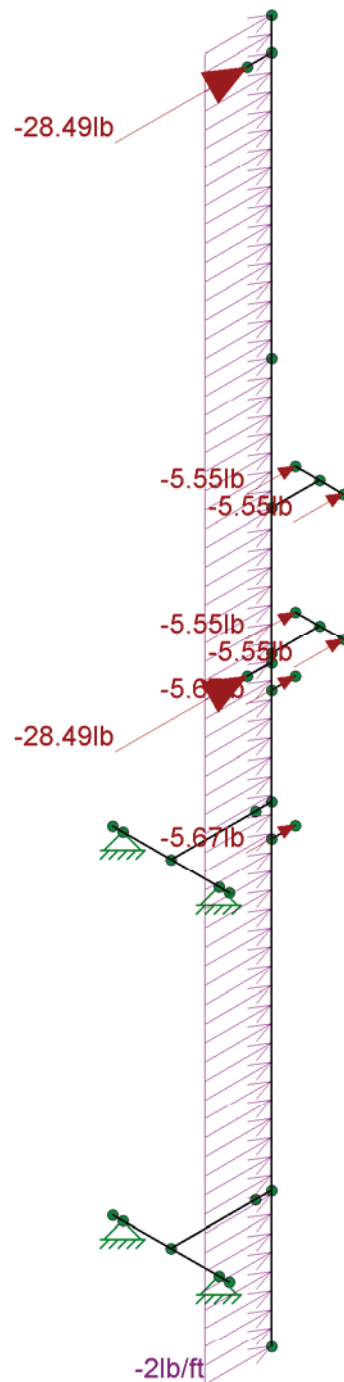
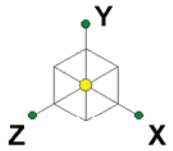
Mount Analysis (Sector Gamma)

Wind Load X

SK - 9

June 2, 2022 at 10:01 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 5, WL.i(0)
Envelope Only Solution

Fullerton Engineering Cons...

AA

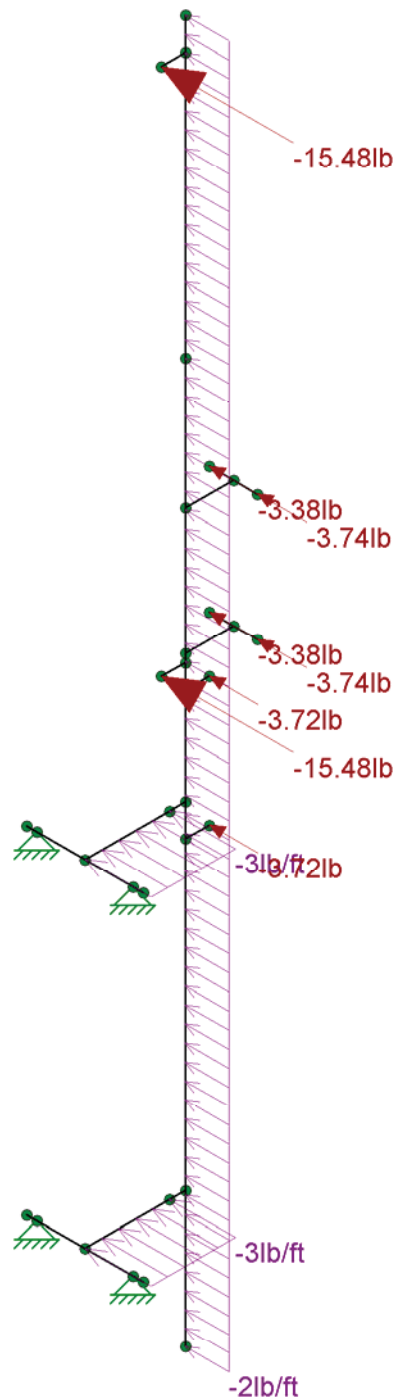
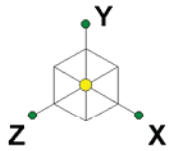
MWMKE00116B

Mount Analysis (Sector Gamma)
Wind Load with Ice Z

SK - 10

June 2, 2022 at 10:01 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 6, WL.i(90)
Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116B

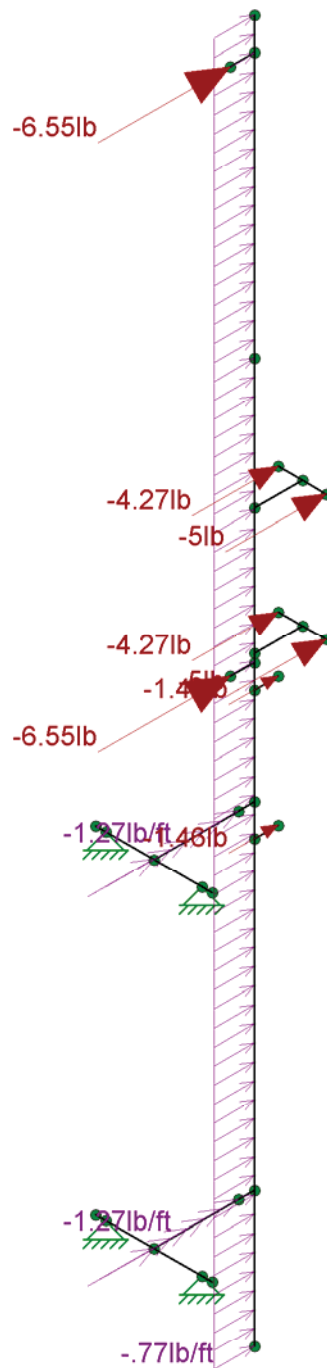
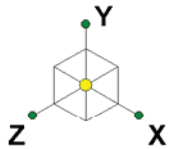
Mount Analysis (Sector Gamma)

Wind Load with Ice X

SK - 11

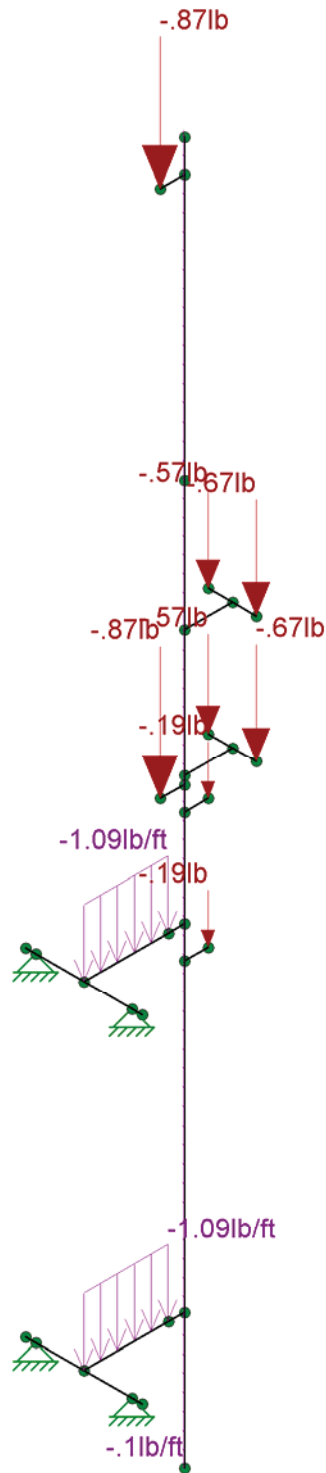
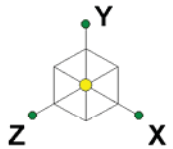
June 2, 2022 at 10:01 AM

MWMKE00120 - Mount Analysis (S...



Loads: BLC 8, EH(0)
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Seismic Load (Lateral)	SK - 12
AA		June 2, 2022 at 10:02 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Loads: BLC 10, EV
Envelope Only Solution

Fullerton Engineering Cons...	Mount Analysis (Sector Gamma) Seismic Load (Vertical)	SK - 13
AA		June 2, 2022 at 10:02 AM
MWMKE00116B		MWMKE00120 - Mount Analysis (S...



Company : Fullerton Engineering Consultants, LLC
 Designer : AA
 Job Number : MWMKE00116B
 Model Name : Mount Analysis (Sector Gamma)

June 2, 2022
 10:04 AM
 Checked By: BK

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver
Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

(Global) Model Settings, Continued

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

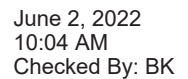
Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torq...	Kyy	Kzz	Cb	Function
1	M1	L3X3X4	14.5	12	12	12	12	12	2.1	2.1		Lateral
2	M2	L3X3X4	14.5	12	12	12	12	12	2.1	2.1		Lateral
3	M3	PIPE 2.5	144	48	48	48	48	48	2.1	2.1		Lateral
4	M14	SBWM	10.5			Lbyy			.65	.65		Lateral
5	M15	SBWM	10.5			Lbyy			.65	.65		Lateral

Material Takeoff

	Material	Size	Pieces	Length[in]	Weight[LB]
1	General				
2	RIGID		12	40	0
3	Total General		12	40	0
4					
5	Hot Rolled Steel				
6	A36 Gr.36	L3X3X4	2	29	11.842
7	A36 Gr.36	SBWM	2	21	17.936



[P:\.....\MWMKE00120 - Mount Analysis (Sector Gamma) .r3d] Page 16
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Load Combinations (Continued)

	Description	So...	P...	S...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...
35	0.9*DL+1.0*EH(0)...	Yes	Y		1	.9	8	1	10	1						
36	0.9*DL+1.0*EH(90)...	Yes	Y		1	.9	9	1	10	-1						
37	0.9*DL+1.0*EH(90)...	Yes	Y		1	.9	9	1	10	1						
38	0.9*DL+1.0*EH(18...	Yes	Y		1	.9	8	-1	10	-1						
39	0.9*DL+1.0*EH(18...	Yes	Y		1	.9	8	-1	10	1						
40	0.9*DL+1.0*EH(27...	Yes	Y		1	.9	9	-1	10	-1						
41	0.9*DL+1.0*EH(27...	Yes	Y		1	.9	9	-1	10	1						
42	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	1						
43	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.866	12	.5				
44	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.5	12	.866				
45	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	12	1						
46	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.5	12	.866				
47	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.866	12	.5				
48	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.1						
49	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.866	12	-.5				
50	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	-.5	12	-.866				
51	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	12	-.1						
52	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.5	12	-.866				
53	1.2*DL+1.5*LM1+1...	Yes	Y		1	1.2	13	1.5	11	.866	12	-.5				

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-in]	LC	MY [lb-in]	LC	MZ [lb-in]	LC
1	N20	max	46.155	11	496.353	13	188.413	7	0	53	0	53	0	53
2		min	-45.676	5	-258.408	7	-325.429	13	0	1	0	1	0	1
3	N21	max	46.155	11	495.484	3	189.428	9	0	53	0	53	0	53
4		min	-45.676	5	-259.277	9	-324.414	3	0	1	0	1	0	1
5	N27	max	227.497	5	835.703	6	771.33	13	0	53	0	53	0	53
6		min	-227.976	11	-652.485	12	-634.437	7	0	1	0	1	0	1
7	N28	max	227.497	5	839.875	10	770.315	3	0	53	0	53	0	53
8		min	-227.976	11	-648.312	4	-635.451	9	0	1	0	1	0	1
9	Totals:	max	363.641	5	824.364	14	601.315	2						
10		min	-363.641	11	311.216	38	-601.319	8						

Envelope AISC 15th(360-16): LRFD Steel Code Checks

Member	Shape	Code Check	Loc[in]	LC	Shear Ch...	Loc[in]	Dir	LC	phi*Pnc	phi*Pnt	phi*Mn	phi*Mn	Cb	Eqn
1	M3	PIPE 2.5	.516	60	2	.045	58.5	2	28468.4...	50715	43155	43155	1	H1-1b
2	M15	SBWM	.384	0	9	.136	4.375	z	1082203.8...	97588.8	31644.4...	356724	1.531	H1-1b
3	M14	SBWM	.327	0	2	.058	5.031	z	1382203.8...	97588.8	31644.4...	356724	1.495	H1-1b
4	M2	L3X3X4	.221	7.25	12	.058	13.1...	y	1042313.8...	46656	20257.6...	45068.9...	1	H2-1
5	M1	L3X3X4	.113	7.25	13	.034	1.359	y	1342313.8...	46656	20257.6...	45068.9...	1	H2-1

Max. Stress Ratio < 1
 Members are adequate

Wall Mount Connection Calculations (Sector Gamma)

New wall mounts will be attached to existing parapet wall via Thru-Bolts 1/2" Ø

Maximum Reactions from Risa Mount Analysis per one thru-bolt:

3D Envelope Joint Reactions														
	Joint		X [lb]	L...	Y [lb]	L...	Z [lb]	L...	MX [lb-i...	L...	MY [lb-i...	L...	MZ [lb-i...	L...
1	N20	max	46.155	11	496.353	13	188.413	7	0	53	0	53	0	53
2		min	-45.676	5	-258.408	7	-325.429	13	0	1	0	1	0	1
3	N21	max	46.155	11	495.484	3	189.428	9	0	53	0	53	0	53
4		min	-45.676	5	-259.277	9	-324.414	3	0	1	0	1	0	1
5	N27	max	227.497	5	835.703	6	771.33	13	0	53	0	53	0	53
6		min	-227.976	11	-652.485	12	-634.437	7	0	1	0	1	0	1
7	N28	max	227.497	5	839.875	10	770.315	3	0	53	0	53	0	53
8		min	-227.976	11	-648.312	4	-635.451	9	0	1	0	1	0	1
9	Totals:	max	363.641	5	824.364	14	601.315	2						
10		min	-363.641	11	311.216	38	-601.319	8						

$$X := 227.976 \text{ lbf}$$

Maximum Factored Reaction - X direction

$$Y := 839.875 \text{ lbf}$$

Maximum Factored Reaction - Y direction

$$Z := 635.451 \text{ lbf}$$

Maximum Factored Reaction - Z direction

$$P_t := Z$$

$$P_t = 635.45 \text{ lbf}$$

Factored Tensile Force

$$P_v := \sqrt{X^2 + Y^2}$$

$$P_v = 870.27 \text{ lbf}$$

Factored Shear Force

$$d_b := 0.5 \text{ in}$$

Diameter of thru-bolt

$$A_b := 0.25\pi \cdot d_b^2$$

$$A_b = 0.2 \text{ in}^2$$

Area of thru-bolt

$$P_{t_bolt} := P_t$$

$$P_{t_bolt} = 635.45 \text{ lbf}$$

Tension at each thru-bolt

$$P_{v_bolt} := P_v$$

$$P_{v_bolt} = 870.27 \text{ lbf}$$

Shear at each thru-bolt

Tensile and Shear Strength of Thru-Bolts

$$F_{nt} := 45 \text{ ksi}$$

Nominal tensile strength per AISC
 360, Table J3.2

$$F_{nv} := 27 \text{ ksi}$$

Nominal shear strength per AISC
 360, Table J3.2

$$\phi_{bolt} := 0.75$$

Resistance Factor (LRFD - AISC
 360, Section J3-6)

$$R_{nt} := \phi_{bolt} \cdot F_{nt} \cdot A_b$$

$$R_{nt} = 6.63 \cdot \text{kip}$$

Design Nominal Tensile Strength
 (AISC 360, Section J3-1)

$$R_{nv} := \phi_{bolt} \cdot F_{nv} \cdot A_b$$

$$R_{nv} = 3.98 \cdot \text{kip}$$

Design Nominal Shear Strength
 (AISC 360, Section J3-1)

$$\frac{P_{t_bolt}}{R_{nt}} = 9.59\% \quad < 30\%$$

$$\frac{P_{v_bolt}}{R_{nv}} = 21.89\% \quad < 30\%$$

Check = "Thru-Bolts are adequate. Effects of combined stress don't need to be investigated because ratio of either tension or shear is under 30%"

New Charles HEX Cube-PM639155N4 Cabinet

Height

height := 74in

Width/Diameter

width := 32in

Depth/Diameter

depth := 32in

Weight

weight := 1067.14lbf

Shape

☒ Flat
☐ Round



$DL_{11} := \text{weight}$

$DL_{11} = 1067.14 \text{ lbf}$

Dead Load

$F_{F11} := F_F$

$F_{F11} = 547.55 \text{ lbf}$

Wind Load on Front Face

$F_{S11} := F_S$

$F_{S11} = 547.55 \text{ lbf}$

Wind Load on Side Face

$DL_{11i} := DL_{ice}$

$DL_{11i} = 468.11 \text{ lbf}$

Dead Load Ice

$F_{F11i} := F_F$

$F_{F11i} = 87.74 \text{ lbf}$

Wind Load with Ice on Front Face

$F_{S11i} := F_S$

$F_{S11i} = 87.74 \text{ lbf}$

Wind Load with Ice on Side Face

$F_{F11m} := F_{Fm}$

$F_{F11m} = 43.04 \text{ lbf}$

Wind Load (maintenance) on Front Face

$F_{S11m} := F_{Sm}$

$F_{S11m} = 43.04 \text{ lbf}$

Wind Load (maintenance) on Side Face

$F_{Eh11} := \text{weight} \cdot A_s \cdot C_s$

$F_{Eh11} = 142.46 \text{ lbf}$

Lateral Earthquake Load

$F_{Ev11} := 0.2 \cdot S_{DS} \cdot \text{weight}$

$F_{Ev11} = 19 \text{ lbf}$

Vertical Earthquake Load

New Charles CFIT-PF2020DSH1 Fiber Telco Enclosure

Height

height := 20in

Width/Diameter

width := 20in

Depth/Diameter

depth := 9in

Weight

weight := 20lbf

Shape



DL₁₁ := weight

$$\frac{DL_{11}}{2} = 10 \text{ lbf}$$

Dead Load

F_{F11} := F_F

$$\frac{F_{F11}}{2} = 46.25 \text{ lbf}$$

Wind Load on Front Face

F_{S11} := F_S

$$\frac{F_{S11}}{2} = 20.81 \text{ lbf}$$

Wind Load on Side Face

DL_{11i} := DL_{ice}

$$\frac{DL_{11i}}{2} = 29.44 \text{ lbf}$$

Dead Load Ice

F_{F11i} := F_F

$$\frac{F_{F11i}}{2} = 8.68 \text{ lbf}$$

Wind Load with Ice on Front Face

F_{S11i} := F_S

$$\frac{F_{S11i}}{2} = 4.56 \text{ lbf}$$

Wind Load with Ice on Side Face

F_{F11m} := F_{Fm}

$$\frac{F_{F11m}}{2} = 3.64 \text{ lbf}$$

Wind Load (maintenance) on Front Face

F_{S11m} := F_{Sm}

$$\frac{F_{S11m}}{2} = 1.64 \text{ lbf}$$

Wind Load (maintenance) on Side Face

F_{Eh11} := weight · A_S · C_s

$$\frac{F_{Eh11}}{2} = 1.34 \text{ lbf}$$

Lateral Earthquake Load

F_{Ev11} := 0.2 · S_{DS} · weight

$$\frac{F_{Ev11}}{2} = 0.18 \text{ lbf}$$

Vertical Earthquake Load

New Raycap PPC RDIAC-2465-P-240-MTS

Height

height := 39in

Width/Diameter

width := 22.9in

Depth/Diameter

depth := 12.6in

Weight

weight := 80lbf

Shape



DL₁₁ := weight

$$\frac{DL_{11}}{2} = 40 \text{ lbf}$$

Dead Load

F_{F11} := F_F

$$\frac{F_{F11}}{2} = 103.25 \text{ lbf}$$

Wind Load on Front Face

F_{S11} := F_S

$$\frac{F_{S11}}{2} = 58.07 \text{ lbf}$$

Wind Load on Side Face

DL_{11i} := DL_{ice}

$$\frac{DL_{11i}}{2} = 59.78 \text{ lbf}$$

Dead Load Ice

F_{F11i} := F_F

$$\frac{F_{F11i}}{2} = 17.77 \text{ lbf}$$

Wind Load with Ice on Front Face

F_{S11i} := F_S

$$\frac{F_{S11i}}{2} = 10.82 \text{ lbf}$$

Wind Load with Ice on Side Face

F_{F11m} := F_{Fm}

$$\frac{F_{F11m}}{2} = 8.12 \text{ lbf}$$

Wind Load (maintenance) on Front Face

F_{S11m} := F_{Sm}

$$\frac{F_{S11m}}{2} = 4.61 \text{ lbf}$$

Wind Load (maintenance) on Side Face

F_{Eh11} := weight · A_S · C_s

$$\frac{F_{Eh11}}{2} = 5.34 \text{ lbf}$$

Lateral Earthquake Load

F_{Ev11} := 0.2 · S_{DS} · weight

$$\frac{F_{Ev11}}{2} = 0.71 \text{ lbf}$$

Vertical Earthquake Load

C8x11.5:

Height

height := 120in

Width/Diameter

width := 8in

Depth/Diameter

depth := 2.26in

Weight

weight := 11.5plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 31 \cdot \text{plf}$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 10 \cdot \text{plf}$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1_linear}} := \frac{D_{L_{ice}}}{\text{height}}$$

$$D_{L_{ice.M1_linear}} = 10.73 \cdot \text{plf}$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 6 \cdot \text{plf}$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 3 \cdot \text{plf}$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 308.3 \cdot \text{lbf}$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 104.51 \cdot \text{lbf}$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 56.88 \cdot \text{lbf}$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 34.67 \cdot \text{lbf}$$

Wind Load with Ice on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 107.28 \cdot \text{lbf}$$

Total Weight of Ice

$$W_{tM1} := \text{Weight}_{\text{total}}$$

$$W_{tM1} = 115 \cdot \text{lbf}$$

Total Weight of member

$$F_{EhM1} := \text{Weight}_{\text{total}} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 15.35 \cdot \text{lbf}$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{\text{height}}$$

$$F_{EhM1_linear} = 1.54 \cdot \text{plf}$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot \text{Weight}_{\text{total}}$$

$$F_{EvM1} = 2.05 \cdot \text{lbf}$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{\text{height}}$$

$$F_{EvM1_linear} = 0.2 \cdot \text{plf}$$

Linear Vertical Earthquake Load

C12x20.7:

Height

height := 36in

Width/Diameter

width := 12in

Depth/Diameter

depth := 2.94in

Weight

weight := 20.7plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 34 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 11 \cdot plf$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1}_linear} := \frac{D_{L_{ice}}}{height}$$

$$D_{L_{ice.M1}_linear} = 13.05 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 6 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 3 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 101.74 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 32.12 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 19.28 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 8.87 \cdot lbf$$

Wind Load with Ice on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 39.16 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 62.1 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 8.29 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 2.76 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 1.11 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.37 \cdot plf$$

Linear Vertical Earthquake Load

L2x2x1/4:

Height

height := 96in

Width/Diameter

width := 2in

Depth/Diameter

depth := 2in

Weight

weight := 3.19plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 9 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 9 \cdot plf$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1_linear}} := \frac{D_{L_{ice}}}{height}$$

$$D_{L_{ice.M1_linear}} = 10.23 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 3 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 3 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 73.99 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 73.99 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 24.97 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 24.97 \cdot lbf$$

Wind Load with Ice on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 81.83 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 25.52 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 3.41 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 0.43 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 0.45 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.06 \cdot plf$$

Linear Vertical Earthquake Load

L4x4x1/4:

Height

height := 120in

Width/Diameter

width := 4in

Depth/Diameter

depth := 4in

Weight

weight := 6.60plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 18 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 18 \cdot plf$$

Linear Wind Load on Side Face

$$D_{Lice.M1_linear} := \frac{D_{Lice}}{height}$$

$$D_{Lice.M1_linear} = 14.14 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 4 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 4 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 184.98 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 184.98 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 41.4 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 41.4 \cdot lbf$$

Wind Load with Ice on Side Face

$$D_{Lice.M1} := D_{Lice}$$

$$D_{Lice.M1} = 141.37 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 66 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 8.81 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 0.88 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 1.17 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.12 \cdot plf$$

Linear Vertical Earthquake Load

P1000 Unistrut:

Height

height := 58in

Width/Diameter

width := 1.625in

Depth/Diameter

depth := 1.625in

Weight

weight := 1.52plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 8 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 8 \cdot plf$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1_linear}} := \frac{D_{L_{ice}}}{height}$$

$$D_{L_{ice.M1_linear}} = 9.59 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 3 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 3 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 36.32 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 36.32 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 12.59 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 12.59 \cdot lbf$$

Wind Load with Ice on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 46.36 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 7.35 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 0.98 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 0.2 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 0.13 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.03 \cdot plf$$

Linear Vertical Earthquake Load

H-Frame Vertical Pipe

$$H_{pipe} := 5\text{-ft}$$

$$Pipe := 3.0$$



$$W_{tp2} := \text{Weight}_{total}$$

$$W_{tp2} = 37.9 \text{ lbf}$$

Total Weight of Pipe

$$F_{p2} := F_p$$

$$F_{p2} = 41.49 \text{ lbf}$$

Wind Load

$$F_{p2_linear} := F_{p_linear}$$

$$F_{p2_linear} = 8 \cdot \text{plf}$$

Linear Wind Load on Pipe

$$D_{Lice,p2} := D_{Lice}$$

$$D_{Lice,p2} = 49.43 \text{ lbf}$$

Total Weight of Ice

$$D_{Lice,p2_linear} := \frac{D_{Lice}}{H_{pipe}}$$

$$D_{Lice,p2_linear} = 9.89 \cdot \text{plf}$$

Linear Ice Load on Pipe

$$F_{p2i} := F_{pi}$$

$$F_{p2i} = 9.71 \text{ lbf}$$

Wind Load with Ice

$$F_{p2i_linear} := F_{pi_linear}$$

$$F_{p2i_linear} = 2 \cdot \text{plf}$$

Linear Wind Load with Ice on Pipe

$$F_{Ehp2} := \text{Weight}_{total} \cdot A_s \cdot C_s$$

$$F_{Ehp2} = 5.06 \text{ lbf}$$

Total Lateral Earthquake Load

$$F_{Ehp2_linear} := \frac{F_{Ehp2}}{H_{pipe}}$$

$$F_{Ehp2_linear} = 1.01 \cdot \text{plf}$$

Linear Lateral Earthquake Load

$$F_{Evp2} := 0.2 \cdot S_{DS} \cdot \text{Weight}_{total}$$

$$F_{Evp2} = 0.67 \text{ lbf}$$

Total Vertical Earthquake Load

$$F_{Evp2_linear} := \frac{F_{Evp2}}{H_{pipe}}$$

$$F_{Evp2_linear} = 0.13 \cdot \text{plf}$$

Linear Vertical Earthquake Load

PL3/8x3:

Height

height := 120in

Width/Diameter

width := 3in

Depth/Diameter

depth := 0.375in

Weight

weight := 3.83plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 14 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 2 \cdot plf$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1_linear}} := \frac{D_{L_{ice}}}{height}$$

$$D_{L_{ice.M1_linear}} = 7 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 4 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 2 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 138.74 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 17.34 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 37.53 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 23.58 \cdot lbf$$

Wind Load with Ice on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 69.97 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 38.3 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 5.11 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 0.51 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 0.68 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.07 \cdot plf$$

Linear Vertical Earthquake Load

W12x22:

Height

height := 96in

Width/Diameter

width := 12.3in

Depth/Diameter

depth := 4in

Weight

weight := 22plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 41 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 18 \cdot plf$$

Linear Wind Load on Side Face

$$D_{L_{ice.M1}_linear} := \frac{D_{L_{ice}}}{height}$$

$$D_{L_{ice.M1}_linear} = 14.29 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 7 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 4 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 324.64 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 145.52 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 56.79 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 31.2 \cdot lbf$$

Wind Load with Ice on Side Face

$$D_{L_{ice.M1}} := D_{L_{ice}}$$

$$D_{L_{ice.M1}} = 114.31 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 176 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 23.5 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 2.94 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

$$F_{EvM1} = 3.13 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

$$F_{EvM1_linear} = 0.39 \cdot plf$$

Linear Vertical Earthquake Load

W10x22:

Height

height := 288in

Width/Diameter

width := 10.2in

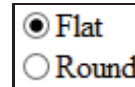
Depth/Diameter

depth := 5.75in

Weight

weight := 22plf

Shape



$$F_{FM1_linear} := F_{F_linear}$$

$$F_{FM1_linear} = 47 \cdot plf$$

Linear Wind Load on Front Face

$$F_{SM1_linear} := F_{S_linear}$$

$$F_{SM1_linear} = 27 \cdot plf$$

Linear Wind Load on Side Face

$$DL_{ice.M1_linear} := \frac{DL_{ice}}{height}$$

$$DL_{ice.M1_linear} = 17.45 \cdot plf$$

Linear Ice Load on Member

$$F_{FiM1_linear} := F_{Fi_linear}$$

$$F_{FiM1_linear} = 8 \cdot plf$$

Linear Wind Load with Ice on Front Face

$$F_{SiM1_linear} := F_{Si_linear}$$

$$F_{SiM1_linear} = 6 \cdot plf$$

Linear Wind Load with Ice on Side Face

$$F_{FM1} := F_F$$

$$F_{FM1} = 1132.09 \cdot lbf$$

Wind Load on Front Face

$$F_{SM1} := F_S$$

$$F_{SM1} = 638.19 \cdot lbf$$

Wind Load on Side Face

$$F_{FiM1} := F_{Fi}$$

$$F_{FiM1} = 198.5 \cdot lbf$$

Wind Load with Ice on Front Face

$$F_{SiM1} := F_{Si}$$

$$F_{SiM1} = 140.03 \cdot lbf$$

Wind Load with Ice on Side Face

$$DL_{ice.M1} := DL_{ice}$$

$$DL_{ice.M1} = 418.79 \cdot lbf$$

Total Weight of Ice

$$W_{tM1} := Weight_{total}$$

$$W_{tM1} = 528 \cdot lbf$$

Total Weight of member

$$F_{EhM1} := Weight_{total} \cdot A_s \cdot C_s$$

$$F_{EhM1} = 70.49 \cdot lbf$$

Total Lateral Earthquake Load

$$F_{EhM1_linear} := \frac{F_{EhM1}}{height}$$

$$F_{EhM1_linear} = 2.94 \cdot plf$$

Linear Lateral Earthquake Load

$$F_{EvM1} := 0.2 \cdot S_{DS} \cdot Weight_{total}$$

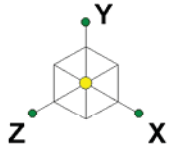
$$F_{EvM1} = 9.4 \cdot lbf$$

Total Vertical Earthquake Load

$$F_{EvM1_linear} := \frac{F_{EvM1}}{height}$$

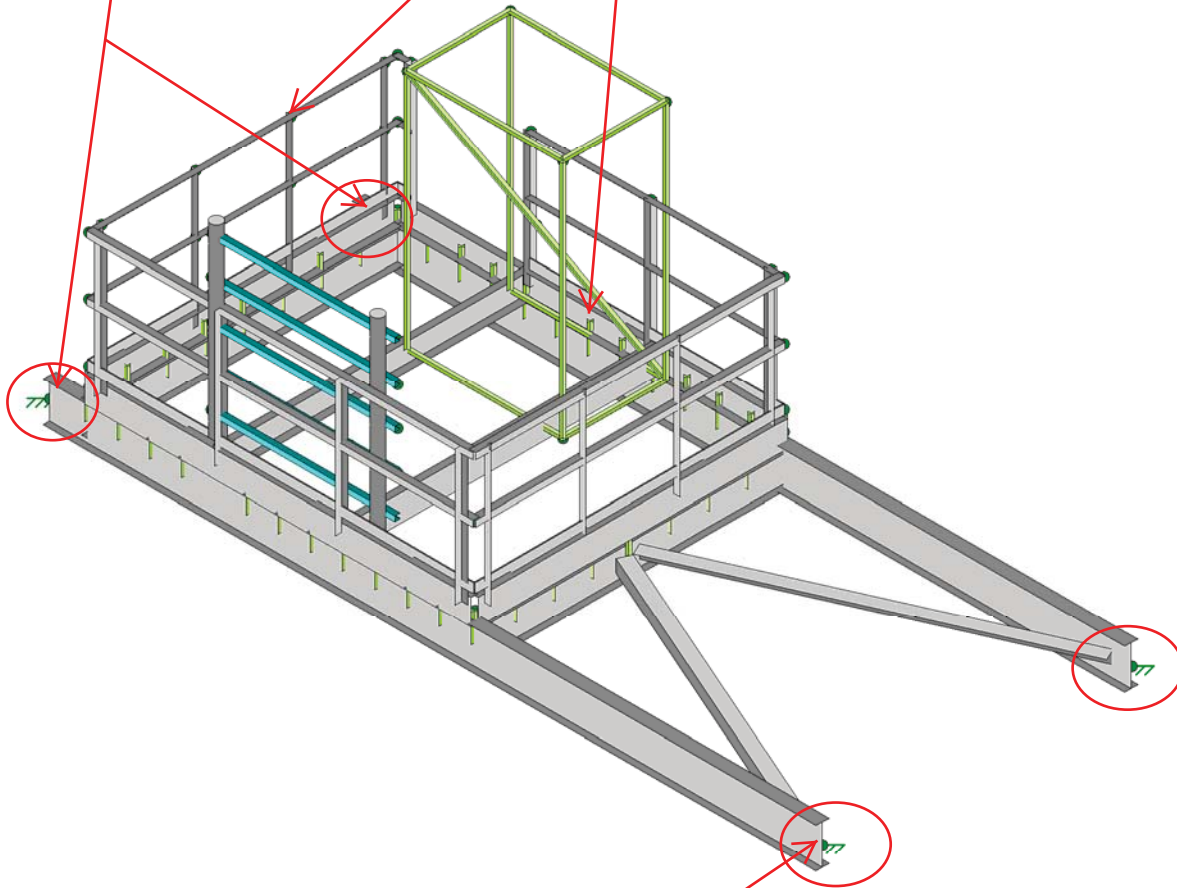
$$F_{EvM1_linear} = 0.39 \cdot plf$$

Linear Vertical Earthquake Load



New attachment to elevated roof wall.
See pages 106-107 of this report.

New Equipment Platform
w/ Handrail



New attachment to parapet wall.
See pages 106-107 of this report.

Envelope Only Solution

Fullerton Engineering Cons...

AA

MWMKE00116

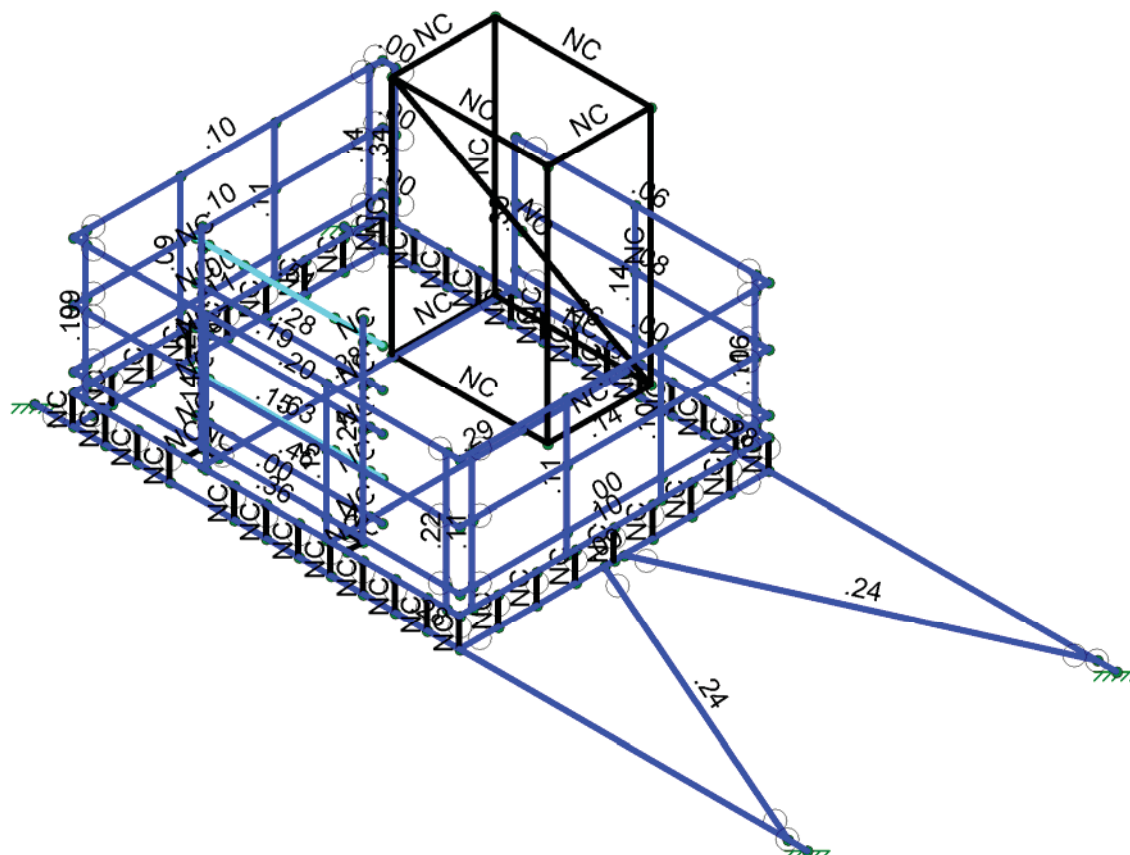
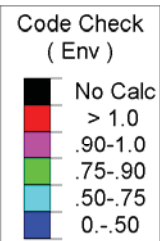
Equipment Platform

Rendered

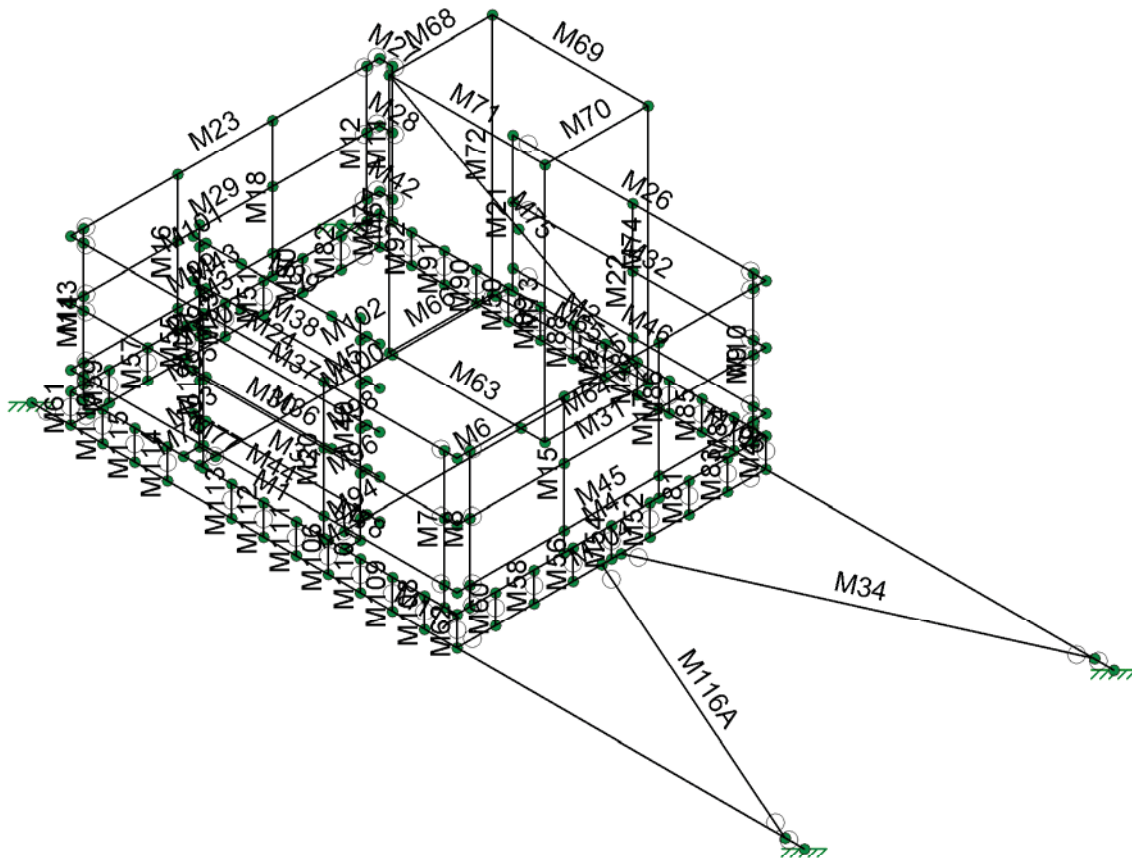
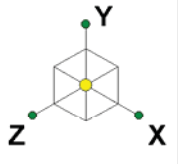
SK - 2

June 2, 2022 at 1:51 PM

MWMKE00116 - Equipment Platfor...



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Fullerton Engineering Cons...

AA

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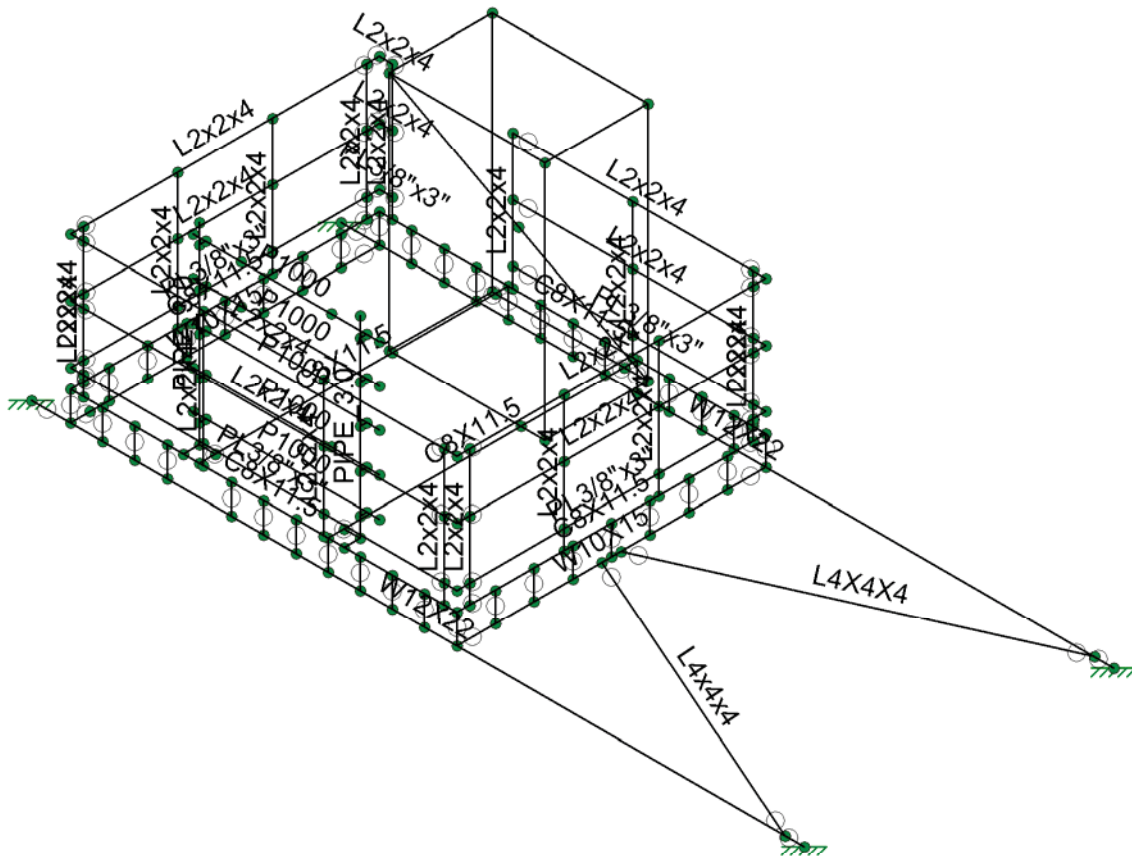
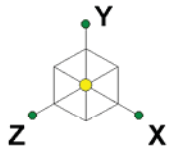
Equipment Platform

Members

SK - 4

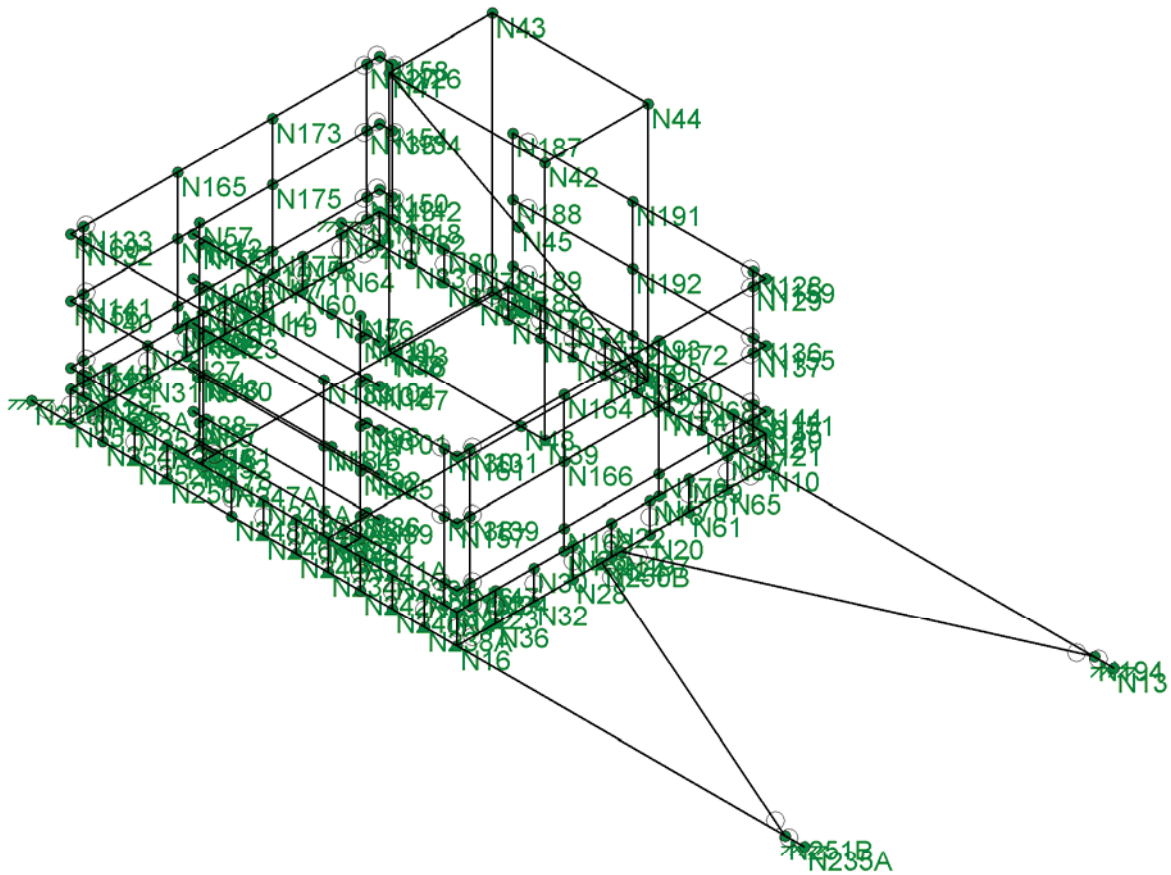
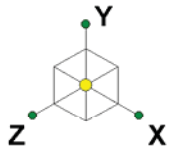
June 2, 2022 at 1:52 PM

MWMKE00116 - Equipment Platfor...



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Fullerton Engineering Cons...	Equipment Platform Shapes	SK - 5
AA		June 2, 2022 at 1:52 PM
MWMKE00116		MWMKE00116 - Equipment Platfor...



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Fullerton Engineering Cons...

AA

MWMKE00116

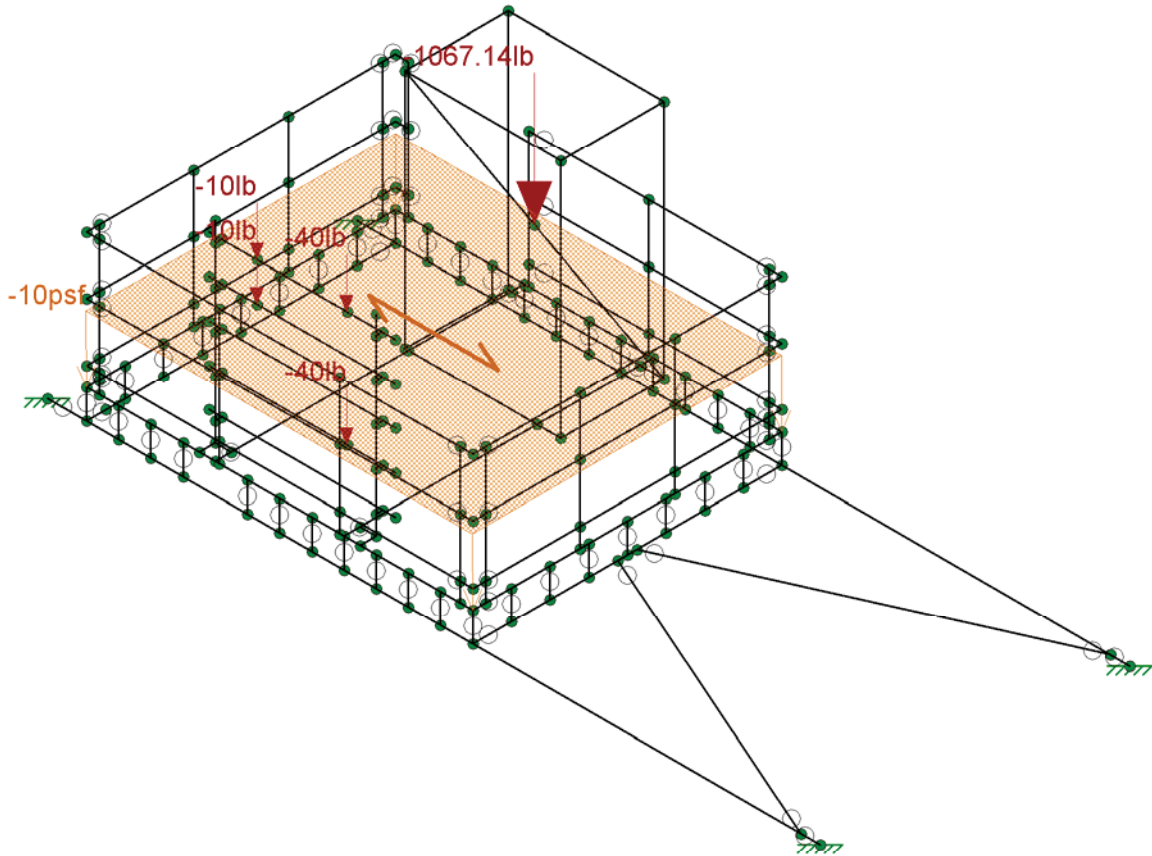
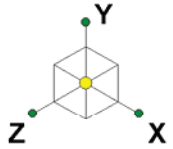
Equipment Platform

Nodes

SK - 6

June 2, 2022 at 1:52 PM

MWMKE00116 - Equipment Platfor...



Loads: BLC 1, DL
Envelope Only Solution

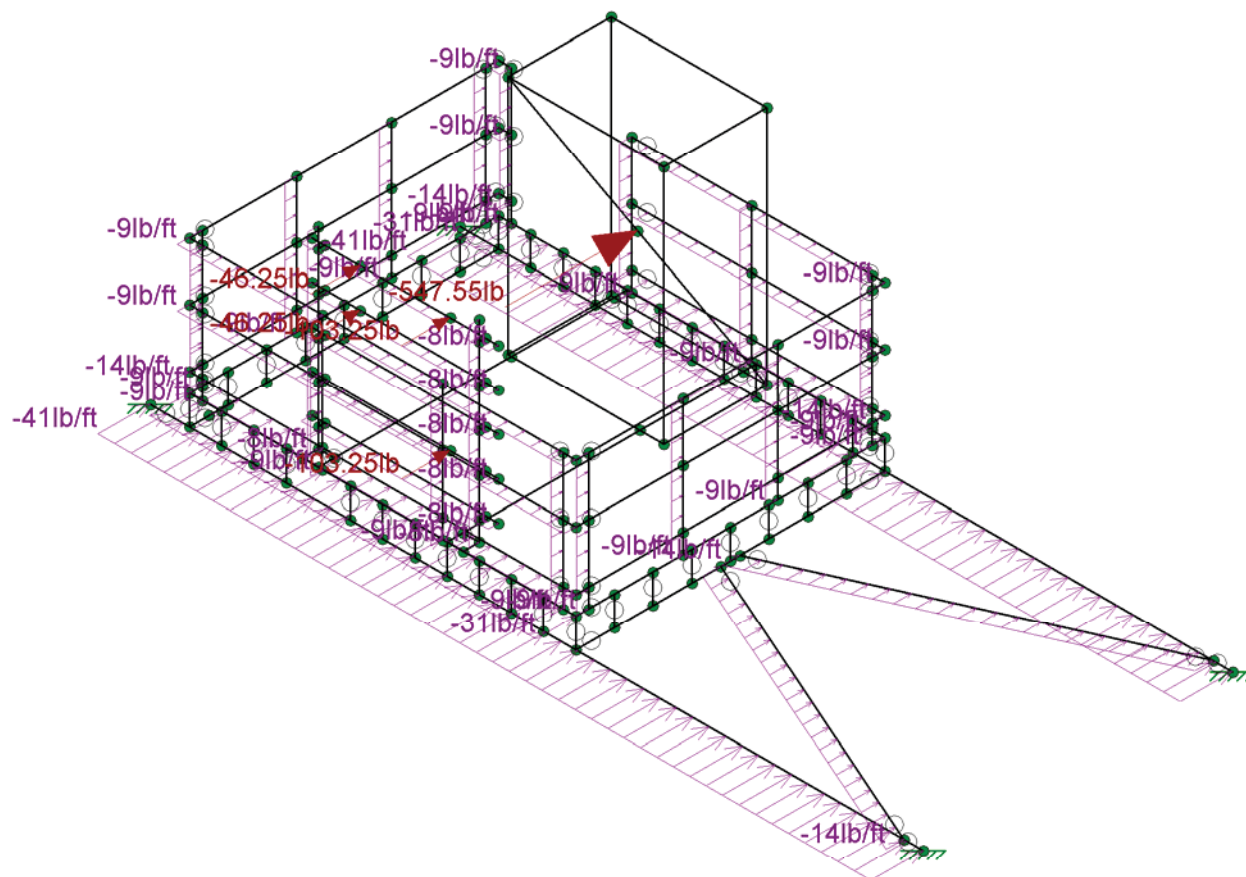
Fullerton Engineering Cons...
AA
MWMKE00116

Equipment Platform
Dead Load

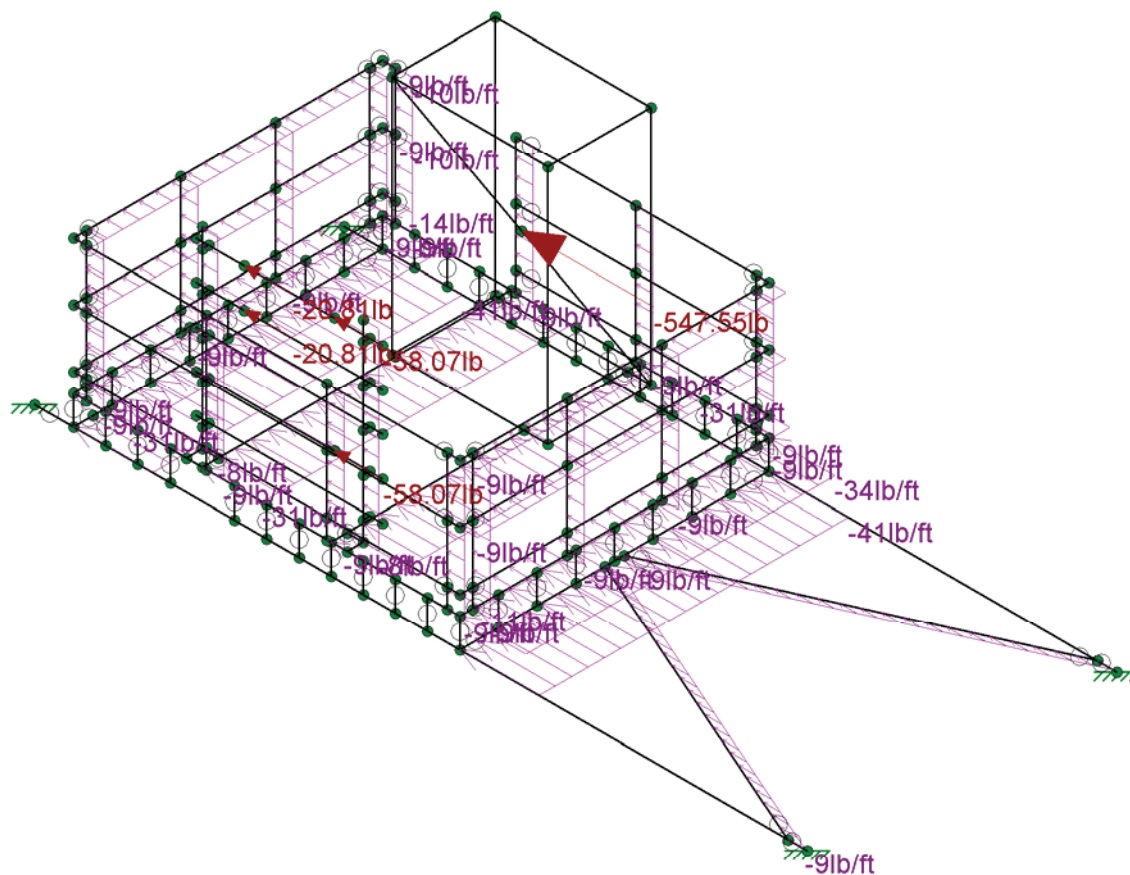
SK - 7

June 2, 2022 at 1:53 PM

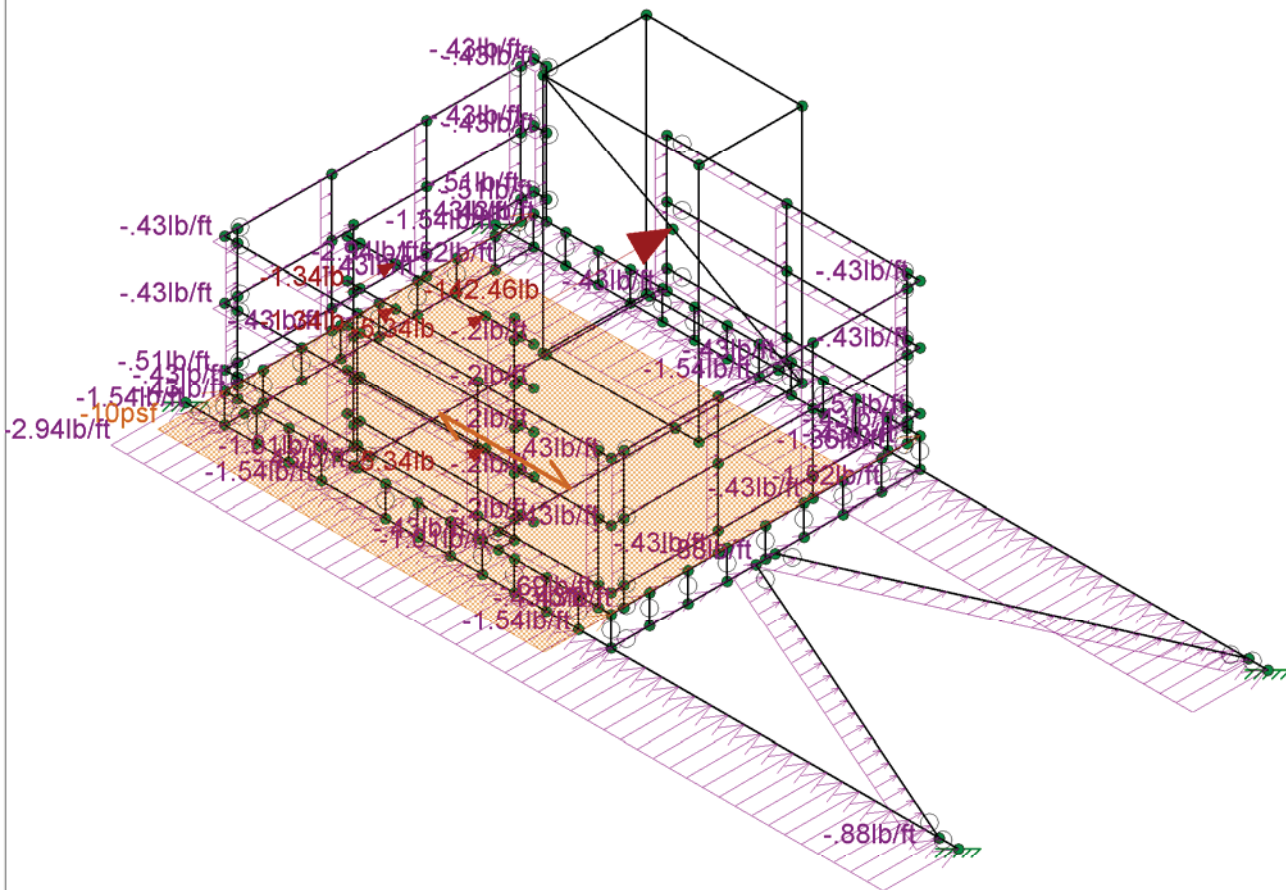
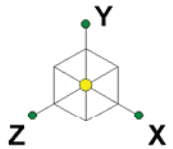
MWMKE00116 - Equipment Platfor...



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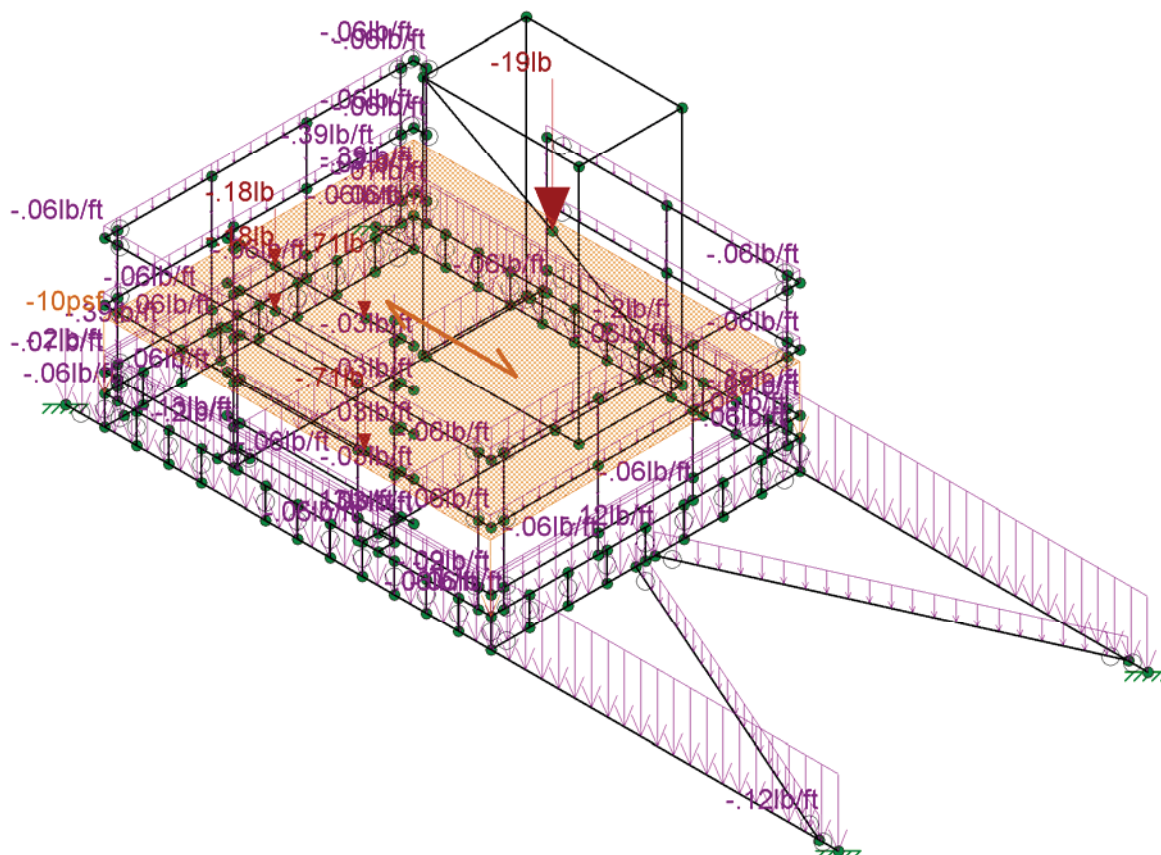


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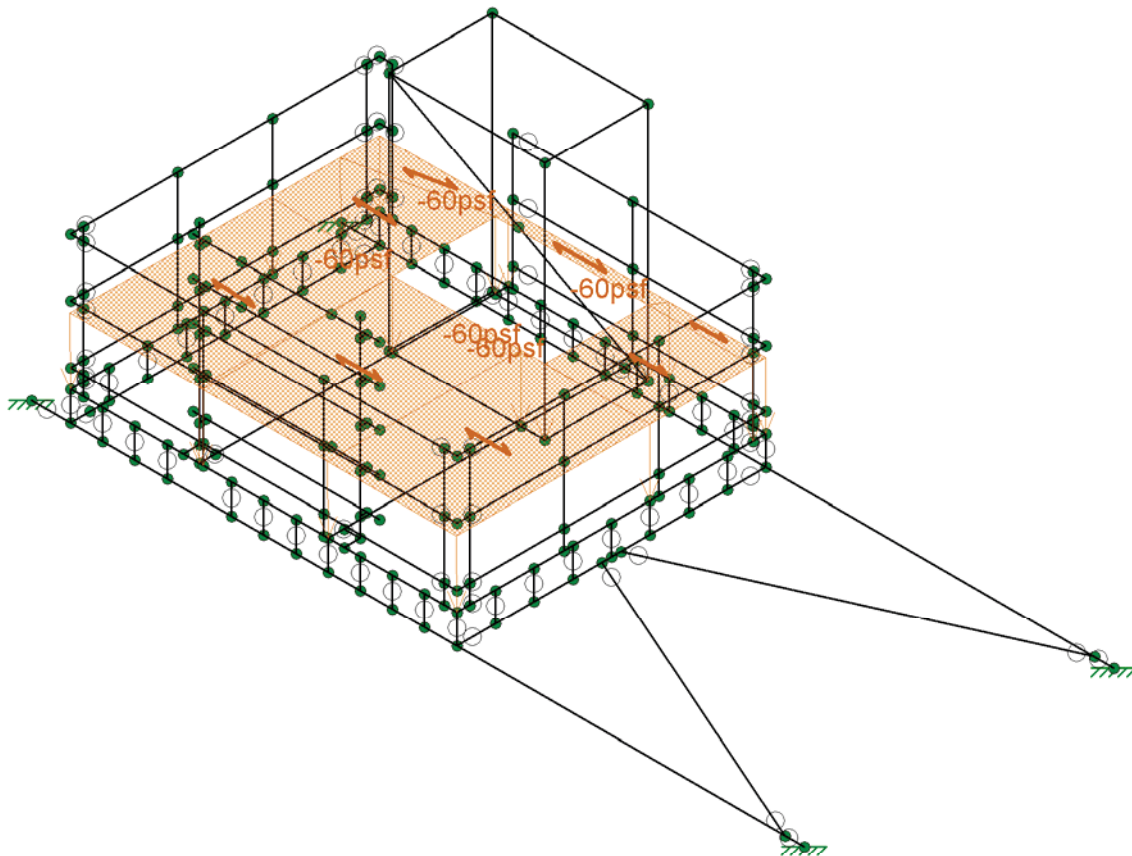
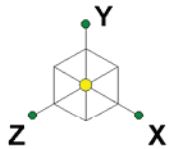


Loads: BLC 4, EH(0)
Envelope Only Solution

Fullerton Engineering Cons...	Equipment Platform Seismic Load (Lateral)	SK - 10
AA		June 2, 2022 at 1:54 PM
MWMKE00116		MWMKE00116 - Equipment Platfor...

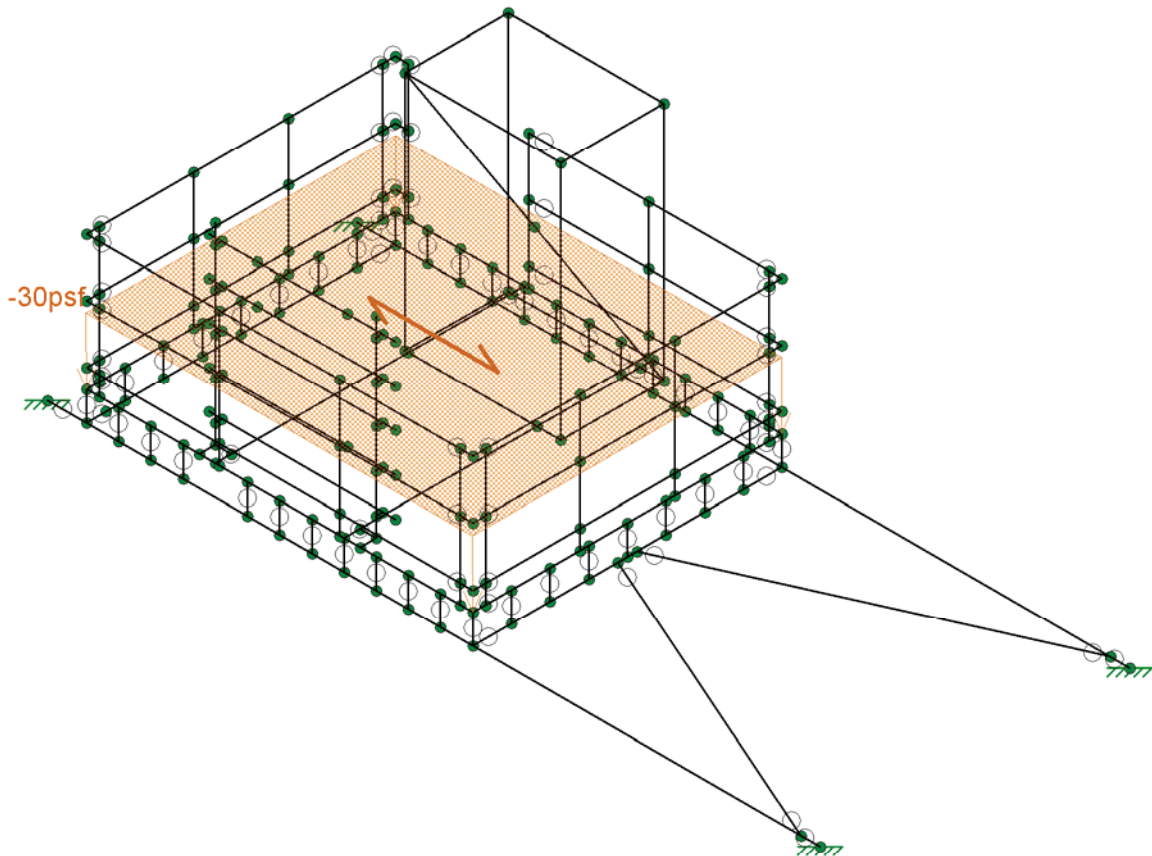
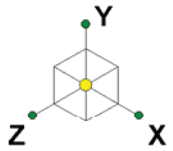


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Loads: BLC 7, LL
Envelope Only Solution

Fullerton Engineering Cons...	Equipment Platform Live Load	SK - 12
AA		June 2, 2022 at 1:55 PM
MWMKE00116		MWMKE00116 - Equipment Platfor...



Loads: BLC 8, SL
Envelope Only Solution

Fullerton Engineering Cons...	Equipment Platform Snow Load	SK - 13
AA		June 2, 2022 at 1:56 PM
MWMKE00116		MWMKE00116 - Equipment Platfor...



Company : Fullerton Engineering Consultants, LLC
 Designer : AA
 Job Number : MWMKE00116
 Model Name : Equipment Platform

June 2, 2022
 1:57 PM
 Checked By: BK

(Global) Model Settings

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation?	Yes
Increase Nailing Capacity for Wind?	Yes
Include Warping?	Yes
Trans Load Btwn Intersecting Wood Wall?	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Include P-Delta for Walls?	Yes
Automatically Iterate Stiffness for Walls?	Yes
Max Iterations for Wall Stiffness	3
Gravity Acceleration (in/sec^2)	386.4
Wall Mesh Size (in)	12
Eigensolution Convergence Tol. (1.E-)	4
Vertical Axis	Y
Global Member Orientation Plane	XZ
Static Solver	Sparse Accelerated
Dynamic Solver	Accelerated Solver
Hot Rolled Steel Code	AISC 15th(360-16): LRFD
Adjust Stiffness?	Yes(Iterative)
RISAConnection Code	AISC 15th(360-16): LRFD
Cold Formed Steel Code	AISI S100-16: LRFD
Wood Code	None
Wood Temperature	< 100F
Concrete Code	None
Masonry Code	None
Aluminum Code	None - Building
Stainless Steel Code	None

(Global) Model Settings, Continued

Number of Shear Regions	4
Region Spacing Increment (in)	4
Biaxial Column Method	Exact Integration
Parame Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections?	Yes
Use Cracked Sections Slab?	No
Bad Framing Warnings?	No
Unused Force Warnings?	Yes
Min 1 Bar Diam. Spacing?	No
Concrete Rebar Set	REBAR SET ASTMA615
Min % Steel for Column	1
Max % Steel for Column	8

Seismic Code	ASCE 7-16
Seismic Base Elevation (in)	Not Entered
Add Base Weight?	Yes
Ct X	.02
Ct Z	.02
T X (sec)	Not Entered
T Z (sec)	Not Entered
R X	3
R Z	3
Ct Exp. X	.75
Ct Exp. Z	.75
SD1	1
SDS	1
S1	1
TL (sec)	5
Risk Cat	I or II
Drift Cat	Other
Om Z	1
Om X	1
Cd Z	1
Cd X	1
Rho Z	1
Rho X	1

Hot Rolled Steel Design Parameters

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torq...	Kyy	Kzz	Cb	Function
1	M1	C8X11.5	120	0	0	0	0	0	.65	.65		Lateral
2	M2	C8X11.5	120	0	0	0	0	0	.65	.65		Lateral
3	M3	C8X11.5	96	0	0	0	0	0	1	1		Lateral
4	M4	C8X11.5	96	0	0	0	0	0	1	1		Lateral
5	M5	C8X11.5	96	0	0	0	0	0	1	1		Lateral
6	M6	C8X11.5	96	0	0	0	0	0	1	1		Lateral
7	M7	L2x2x4	42			Lbyy			.65	.65		Lateral
8	M8	L2x2x4	42			Lbyy			.65	.65		Lateral
9	M9	L2x2x4	42			Lbyy			.65	.65		Lateral
10	M10	L2x2x4	42			Lbyy			.65	.65		Lateral
11	M11	L2x2x4	42			Lbyy			.65	.65		Lateral
12	M12	L2x2x4	42			Lbyy			.65	.65		Lateral
13	M13	L2x2x4	42			Lbyy			.65	.65		Lateral
14	M14	L2x2x4	42			Lbyy			.65	.65		Lateral
15	M15	L2x2x4	42			Lbyy			.65	.65		Lateral
16	M16	L2x2x4	42			Lbyy			.65	.65		Lateral

Hot Rolled Steel Design Parameters (Continued)

	Label	Shape	Length[in]	Lbyy[in]	Lbzz[in]	Lcomp top[in]	Lcomp bot[in]	L-torq...	Kyy	Kzz	Cb	Function
17	M17	L2x2x4	42			Lbyy			.65	.65		Lateral
18	M18	L2x2x4	42			Lbyy			.65	.65		Lateral
19	M19	L2x2x4	42			Lbyy			.65	.65		Lateral
20	M20	L2x2x4	42			Lbyy			.65	.65		Lateral
21	M21	L2x2x4	42			Lbyy			.65	.65		Lateral
22	M22	L2x2x4	42			Lbyy			.65	.65		Lateral
23	M23	L2x2x4	96			Lbyy			1	1		Lateral
24	M24	L2x2x4	120			Lbyy			1	1		Lateral
25	M25	L2x2x4	96			Lbyy			1	1		Lateral
26	M26	L2x2x4	78.667			Lbyy			1	1		Lateral
27	M27	L2x2x4	4			Lbyy			1	1		Lateral
28	M28	L2x2x4	4			Lbyy			1	1		Lateral
29	M29	L2x2x4	96			Lbyy			1	1		Lateral
30	M30	L2x2x4	120			Lbyy			1	1		Lateral
31	M31	L2x2x4	96			Lbyy			1	1		Lateral
32	M32	L2x2x4	78.667			Lbyy			1	1		Lateral
33	M34	L4X4X4	111.485			Lbyy						Lateral
34	M40	PIPE 3.0	60	60	60	60	60	60	2.1	2.1		Lateral
35	M41	PIPE 3.0	60	60	60	60	60	60	2.1	2.1		Lateral
36	M42	PL3/8"x3"	4			Lbyy			1	1		Lateral
37	M43	PL3/8"x3"	96			Lbyy			1	1		Lateral
38	M44	PL3/8"x3"	120			Lbyy			1	1		Lateral
39	M45	PL3/8"x3"	96			Lbyy			1	1		Lateral
40	M46	PL3/8"x3"	78.667			Lbyy			1	1		Lateral
41	M103	W10X15	96	0	0	0	0	0	.8	.8		Lateral
42	M104	W10X15	96	0	0	0	0	0	.8	.8		Lateral
43	M105	W12X22	240	84	84	84	84	84	1	1		Lateral
44	M117	W12X22	240	84	84	84	84	84	1	1		Lateral
45	M116A	L4x4x4	111.485			Lbyy						Lateral

Material Takeoff

	Material	Size	Pieces	Length[in]	Weight[LB]
1	General				
2	RIGID		66	1107.4	0
3	Total General		66	1107.4	0
4					
5	Hot Rolled Steel				
6	A36 Gr.36	C8X11.5	6	624	596.303
7	A36 Gr.36	L2x2x4	26	1461.3	391.177
8	A36 Gr.36	L4X4X4	1	111.5	61.014
9	A36 Gr.36	L4x4x4	1	111.5	61.014
10	A36 Gr.36	PL3/8"x3"	5	394.7	125.903
11	A53 Gr. B	PIPE 3.0	2	120	70.438
12	A992	W10X15	2	192	240.1
13	A992	W12X22	2	480	882
14	Total HR Steel		45	3495	2427.948
15					
16	Cold Formed Steel				
17	A570 Gr.33	P1000	5	290	36.759
18	Total CF Steel		5	290	36.759

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	DL	None		-1		5			1	
2	WL(0)	None				5		69		
3	WL(90)	None				5		64		
4	EH(0)	None				5		98	1	
5	EH(90)	None				5		98	1	
6	EV	None				5		94	1	
7	LL	None							8	
8	SL	None							1	
9	BLC 1 Transient Area Loads	None						35		
10	BLC 8 Transient Area Loads	None						35		
11	BLC 4 Transient Area Loads	None						35		
12	BLC 6 Transient Area Loads	None						35		
13	BLC 5 Transient Area Loads	None						35		
14	BLC 7 Transient Area Loads	None						64		

Load Combinations

	Description	So...	P...	S...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...	BLC Fac...
1	1.4*DL	Yes	Y		1	1.4														
2	1.2*DL + 1.0* WL(0)	Yes	Y		1	1.2	2	1	8	.5										
3	1.2*DL + 1.0*WL(90)	Yes	Y		1	1.2	3	1	8	.5										
4	1.2*DL + 1.0 * WL(180)	Yes	Y		1	1.2	2	-1	8	.5										
5	1.2*DL + 1.0*WL(270)	Yes	Y		1	1.2	3	-1	8	.5										
6	1.2*DL+1.0*EH(0)+...	Yes	Y		1	1.2	4	1	6	1										
7	1.2*DL+1.0*EH(0)+...	Yes	Y		1	1.2	4	1	6	-1										
8	1.2*DL+1.0*EH(90)+...	Yes	Y		1	1.2	5	1	6	1										
9	1.2*DL+1.0*EH(90)+...	Yes	Y		1	1.2	5	1	6	-1										
10	1.2*DL+1.0*EH(180)+...	Yes	Y		1	1.2	4	-1	6	1										
11	1.2*DL+1.0*EH(180)+...	Yes	Y		1	1.2	4	-1	6	-1										
12	1.2*DL+1.0*EH(270)+...	Yes	Y		1	1.2	5	-1	6	1										
13	1.2*DL+1.0*EH(270)+...	Yes	Y		1	1.2	5	-1	6	-1										
14	0.9*DL+1.0*WL(0)	Yes	Y		1	.9	2	1												
15	0.9*DL+1.0*WL(90)	Yes	Y		1	.9	3	1												
16	0.9*DL+1.0*WL(180)	Yes	Y		1	.9	2	-1												
17	0.9*DL+1.0*WL(270)	Yes	Y		1	.9	3	-1												
18	0.9*DL+1.0*EH(0)-...	Yes	Y		1	.9	4	1	6	-1										
19	0.9*DL+1.0*EH(0)-...	Yes	Y		1	.9	4	1	6	1										
20	0.9*DL+1.0*EH(90)-...	Yes	Y		1	.9	5	1	6	-1										
21	0.9*DL+1.0*EH(90)-...	Yes	Y		1	.9	5	1	6	1										
22	0.9*DL+1.0*EH(180)-...	Yes	Y		1	.9	4	-1	6	-1										
23	0.9*DL+1.0*EH(180)-...	Yes	Y		1	.9	4	-1	6	1										
24	0.9*DL+1.0*EH(270)-...	Yes	Y		1	.9	5	-1	6	-1										
25	0.9*DL+1.0*EH(270)-...	Yes	Y		1	.9	5	-1	6	1										
26	1.2*DL+1.6*LL+0.5...	Yes	Y		1	1.2	7	1.6	8	.5										
27	1.2*DL+1.6*LL+0.5...	Yes	Y		1	1.2	7	1.6	2	.5										
28	1.2*DL+1.6*LL+0.5...	Yes	Y		1	1.2	7	1.6	3	.5										
29	1.2*DL+1.6*LL+0.5...	Yes	Y		1	1.2	7	1.6	2	-.5										
30	1.2*DL+1.6*LL+0.5...	Yes	Y		1	1.2	7	1.6	3	-.5										
31	DL	Yes	Y		1	1														

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-in]	LC	MY [lb-in]	LC	MZ [lb-in]	LC
1	N13	max	2770.443	16	2125.02	26	1709.421	2	.545	14	0	31	0	31
2		min	-6767.404	2	504.318	16	-1713.294	4	-5.418	29	0	1	0	1
3	N14	max	4666.039	28	4431.327	26	811.996	14	.014	14	0	31	0	31
4		min	503.969	17	764.569	16	-839.217	4	-.139	29	0	1	0	1
5	N235A	max	3129.864	14	2129.09	26	1698.538	2	5.403	27	0	31	0	31
6		min	-6189.022	4	390.421	14	-1698.425	4	.297	16	0	1	0	1
7	N236A	max	4398.243	28	4521.289	26	464.724	2	.139	27	0	31	0	31
8		min	248.395	17	505.132	14	-434.599	16	.008	16	0	1	0	1
9	N153	max	LOCKED		LOCKED		LOCKED		NC		NC		NC	
10		min	LOCKED		LOCKED		LOCKED		NC		NC		NC	
11	N151	max	LOCKED		LOCKED		LOCKED		NC		NC		NC	
12		min	LOCKED		LOCKED		LOCKED		NC		NC		NC	
13	N152	max	LOCKED		LOCKED		LOCKED		NC		NC		NC	
14		min	LOCKED		LOCKED		LOCKED		NC		NC		NC	
15	N150	max	NC		LOCKED		LOCKED		NC		NC		NC	
16		min	NC		LOCKED		LOCKED		NC		NC		NC	
17	Totals:	max	3618.706	3	13206.726	26	4676.997	2						
18		min	-3618.702	17	3118.264	22	-4676.985	16						

Max. Stress Ratio < 1
 Members are adequate

Envelope AISC 15th(360-16): LRFD Steel Code Checks

Member	Shape	Code	Check	Loc[in]	LC	Shear Ch	Loc[in]	Dir	LC	phi*Pnc	phi*Pnt	phi*Mn	phi*Mn	Cb	Eqn
1	M1	C8X11.5	.365	85	4	.212	10	y	26	109188	109188	40231.28	312012	1.589	H1-1b
2	M11	L2x2x4	.341	0	5	.012	0	z	5	23536.6...	30585.6	8291.204	18922.19	2.578	H2-1
3	M21	L2x2x4	.297	0	2	.011	0	y	2	23536.6...	30585.6	8291.204	18922.19	1.789	H2-1
4	M6	C8X11.5	.290	4	30	.104	0	z	26	109188	109188	40231.28	312012	1.437	H1-1b
5	M5	C8X11.5	.285	92	26	.096	92	z	26	109188	109188	40231.28	312012	1.433	H1-1b
6	M105	W12X22	.280	132.5	27	.065	122.5	y	26	142305...	291600	164700	981699...	1	H1-1b
7	M117	W12X22	.279	132.5	29	.066	122.5	y	26	142305...	291600	164700	981699...	1	H1-1b
8	M2	C8X11.5	.264	120	2	.347	40	y	2	109188	109188	40231.28	312012	1.528	H1-1b
9	M40	PIPE 3.0	.251	0	4	.165	0		4	36138.4	65205	68985	68985	1	H1-1b
10	M41	PIPE 3.0	.242	0	4	.167	0		4	36138.4	65205	68985	68985	1	H1-1b
11	M116A	L4x4x4	.242	54.5...	16	.004	0	y	2	21507.2...	62532	37651.1...	60842.4	1.136	H2-1
12	M34	L4X4X4	.242	56.9...	14	.004	111...	y	4	21507.2...	62532	37651.1...	60842.4	1.136	H2-1
13	M7	L2x2x4	.216	0	30	.065	0	y	2	23536.6...	30585.6	8291.204	18922.19	2.389	H2-1
14	M14	L2x2x4	.192	0	28	.059	0	z	2	23536.6...	30585.6	8291.204	18922.19	2.38	H2-1
15	M24	L2x2x4	.188	60	2	.012	5	z	2	2218.05	30585.6	8291.204	13882.5...	1.151	H2-1
16	M10	L2x2x4	.156	0	5	.069	0	y	2	23536.6...	30585.6	8291.204	18922.19	2.272	H2-1
17	M30	L2x2x4	.151	41.25	2	.027	120	y	30	2218.05	30585.6	8291.204	14540.7...	1.312	H2-1
18	M20	L2x2x4	.147	0	4	.014	0	y	16	23536.6...	30585.6	8291.204	18922.19	2.359	H2-1
19	M19	L2x2x4	.142	0	3	.016	0	y	4	23536.6...	30585.6	8291.204	18922.19	2.708	H2-1
20	M31	L2x2x4	.137	4	30	.032	4	y	5	3465.703	30585.6	8291.204	17715.1...	2.325	H2-1
21	M22	L2x2x4	.136	0	2	.038	0	y	4	23536.6...	30585.6	8291.204	18922.19	1.646	H2-1
22	M12	L2x2x4	.135	0	5	.033	0	y	3	23536.6...	30585.6	8291.204	18399.3...	1.27	H2-1
23	M25	L2x2x4	.125	48	3	.018	4	y	5	3465.703	30585.6	8291.204	14928.9...	1.139	H2-1
24	M18	L2x2x4	.114	0	5	.012	0	y	5	23536.6...	30585.6	8291.204	18922.19	2.32	H2-1
25	M8	L2x2x4	.111	0	4	.037	0	y	5	23536.6...	30585.6	8291.204	18922.19	2.274	H2-1
26	M3	C8X11.5	.108	84	3	.153	96	z	3	109188	109188	40231.28	312012	1.486	H1-1b
27	M15	L2x2x4	.107	0	4	.012	0	y	3	23536.6...	30585.6	8291.204	18922.19	2.579	H2-1
28	M9	L2x2x4	.104	0	4	.037	0	y	5	23536.6...	30585.6	8291.204	18922.19	2.196	H2-1
29	M4	C8X11.5	.103	84	5	.104	62	z	5	109188	109188	40231.28	312012	1.446	H1-1b
30	M23	L2x2x4	.101	53	5	.014	4	z	5	3465.703	30585.6	8291.204	15190.3...	1.206	H2-1
31	M29	L2x2x4	.101	50	5	.020	92	y	27	3465.703	30585.6	8291.204	14888.0...	1.129	H2-1
32	M17	L2x2x4	.096	0	2	.012	0	y	5	23536.6...	30585.6	8291.204	18922.19	2.371	H2-1
33	M13	L2x2x4	.090	0	2	.041	0	y	5	23536.6...	30585.6	8291.204	18922.19	2.252	H2-1
34	M16	L2x2x4	.090	0	4	.013	0	y	5	23536.6...	30585.6	8291.204	18922.19	2.481	H2-1

Envelope AISC 15th(360-16): LRFD Steel Code Checks (Continued)

	Member	Shape	Code Check	Loc[in]	LC	Shear Ch...	Loc[in]	Dir	LC	phi*Pnc ...	phi*Pnt [...]	phi*Mn ...	phi*Mn ...	Cb	Eqn
35	M104	W10X15	.089	51	4	.064	51	z	2	189395...	198450	103500	720000	1.198	H1-1b
36	M32	L2x2x4	.079	40.9...	2	.016	0	y	5	5161.219	30585.6	8291.204	16738.2...	1.44	H2-1
37	M103	W10X15	.071	48	5	.006	96	y	26	189395...	198450	103500	720000	1.109	H1-1b
38	M26	L2x2x4	.060	31.9...	4	.015	78.6...	z	2	5161.219	30585.6	8291.204	16050.85	1.208	H2-1
39	M28	L2x2x4	.003	1.292	5	.000	4	z	16	30414.0...	30585.6	8291.204	18922.19	1.136	H2-1
40	M27	L2x2x4	.000	2	4	.000	4	z	16	30414.0...	30585.6	8291.204	18922.19	1.136	H2-1
41	M42	PL3/8"x3"	.000	0	15	.000	0	z	16	33922.0...	36450	3417.196	27337.5	1.136	H1-1b*
42	M43	PL3/8"x3"	.000	0	31	.000	0	y	31	323.18	36450	3417.196	13620.3	1	H1-1a
43	M44	PL3/8"x3"	.000	0	31	.000	0	y	31	206.835	36450	3417.196	10896.24	1	H1-1a
44	M45	PL3/8"x3"	.000	0	31	.000	0	y	31	323.18	36450	3417.196	13620.3	1	H1-1a
45	M46	PL3/8"x3"	.000	0	31	.000	0	y	31	481.289	36450	3417.196	16621.3...	1	H1-1a

Equipment Platform Connection Calculations

New equipment platform will be attached to existing parapet wall and elevated roof wall via (4) Thru-Bolts 5/8" Ø

Maximum Reactions from Risa Mount Analysis per one thru-bolt:

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [lb-in]	LC	MY [lb-in]	LC	MZ [lb-in]	LC
1	N13	max	2770.443	16	2125.02	26	1709.421	2	.545	14	0	31	0	31
2		min	-6767.404	2	504.318	16	-1713.294	4	-5.418	29	0	1	0	1
3	N14	max	4666.039	28	4431.327	26	811.996	14	.014	14	0	31	0	31
4		min	503.969	17	764.569	16	-839.217	4	-.139	29	0	1	0	1
5	N235A	max	3129.864	14	2129.09	26	1698.538	2	5.403	27	0	31	0	31
6		min	-6189.022	4	390.421	14	-1698.425	4	.297	16	0	1	0	1
7	N236A	max	4398.243	28	4521.289	26	464.724	2	.139	27	0	31	0	31
8		min	248.395	17	505.132	14	-434.599	16	.008	16	0	1	0	1

$$X := \frac{6767.404}{4} \text{ lbf} = 1691.85 \text{ lbf}$$

Maximum Factored Reaction - X direction

$$Y := \frac{2125.02}{4} \text{ lbf} = 531.25 \text{ lbf}$$

Maximum Factored Reaction - Y direction

$$Z := \frac{1713.294}{4} \text{ lbf} = 428.32 \text{ lbf}$$

Maximum Factored Reaction - Z direction

$$P_t := X$$

$$P_t = 1691.85 \text{ lbf}$$

Factored Tensile Force

$$P_v := \sqrt{Y^2 + Z^2}$$

$$P_v = 682.42 \text{ lbf}$$

Factored Shear Force

$$d_b := 0.625 \text{ in}$$

Diameter of thru-bolt

$$A_b := 0.25\pi \cdot d_b^2$$

$$A_b = 0.31 \cdot \text{in}^2$$

Area of thru-bolt

$$P_{t_bolt} := P_t$$

$$P_{t_bolt} = 1691.85 \text{ lbf}$$

Tension at each thru-bolt

$$P_{v_bolt} := P_v$$

$$P_{v_bolt} = 682.42 \text{ lbf}$$

Shear at each thru-bolt

Tensile and Shear Strength of Thru-Bolts

$$F_{nt} := 45 \text{ ksi}$$

Nominal tensile strength per AISC
 360, Table J3.2

$$F_{nv} := 27 \text{ ksi}$$

Nominal shear strength per AISC
 360, Table J3.2

$$\phi_{bolt} := 0.75$$

Resistance Factor (LRFD - AISC
 360, Section J3-6)

$$R_{nt} := \phi_{bolt} \cdot F_{nt} \cdot A_b$$

$$R_{nt} = 10.35 \cdot \text{kip}$$

Design Nominal Tensile Strength
 (AISC 360, Section J3-1)

$$R_{nv} := \phi_{bolt} \cdot F_{nv} \cdot A_b$$

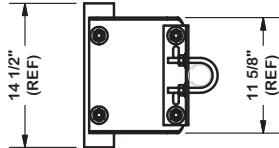
$$R_{nv} = 6.21 \cdot \text{kip}$$

Design Nominal Shear Strength
 (AISC 360, Section J3-1)

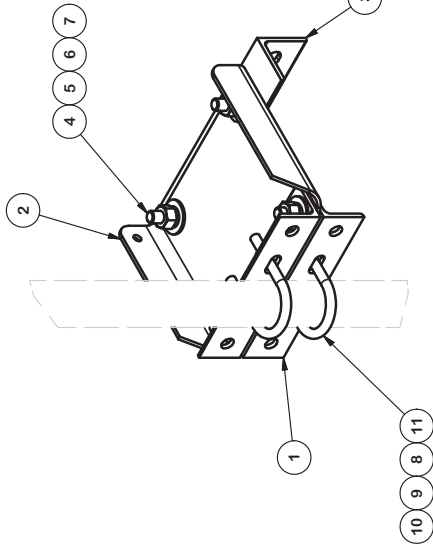
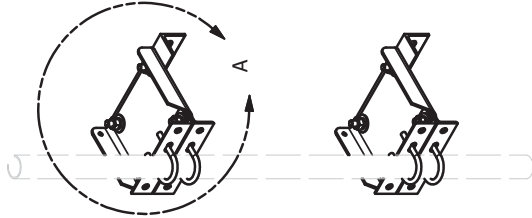
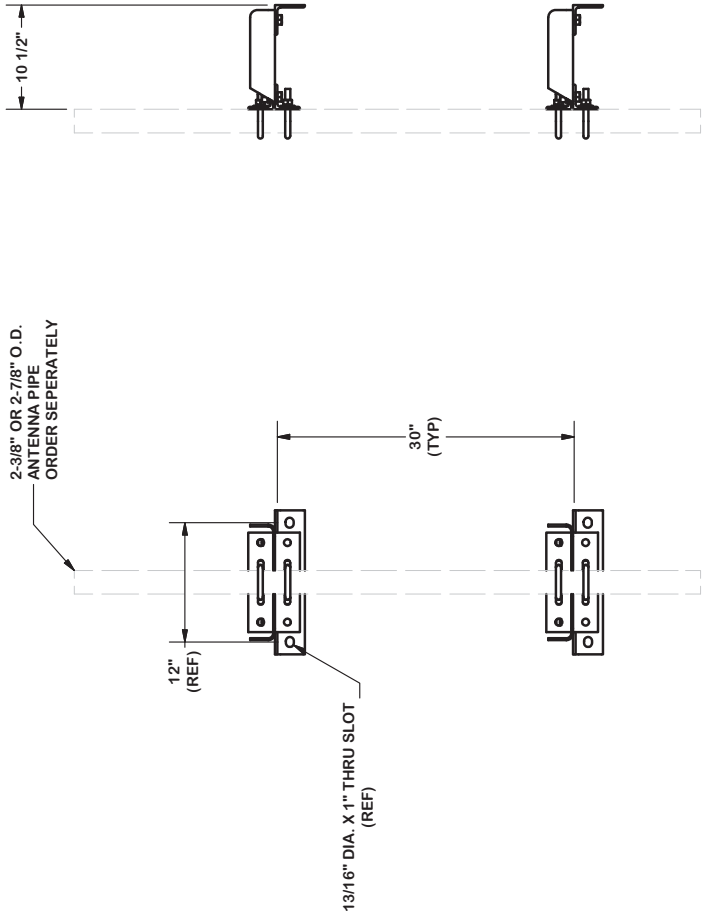
$$\frac{P_{t_bolt}}{R_{nt}} = 16.34\% \quad < 30\%$$

$$\frac{P_{v_bolt}}{R_{nv}} = 10.98\% \quad < 30\%$$

Check = "Thru-Bolts are adequate. Effects of combined stress don't need to be investigated because ratio of either tension or shear is under 30%"



2-3/8" OR 2-7/8" O.D.
ANTENNA PIPE
ORDER SEPERATELY



DETAIL A

PARTS LIST				
ITEM	QTY	PART NO.	PART DESCRIPTION	LENGTH
1	4	X-SLD-A	SLIDER BRACKET ANGLE	10 in
2	2	X-SLD-BP	SLIDER BRACKET BENT PLATE	15 3/4 in
3	2	X-SLD3	SLIDER BRACKET WALL ANGLE	14 1/2 in
4	8	G5802	5/8" x 2" HDG HEX BOLT GR5	2.16
5	8	G58FW	5/8" HDG USS FLATWASHER	0.07
6	8	G58LW	5/8" HDG LOCKWASHER	0.03
7	8	G58NUT	5/8" HDG HEAVY 2H HEX NUT	0.13
8	8	G12FW	1/2" HDG USS FLATWASHER	0.03
9	8	G12LW	1/2" HDG LOCKWASHER	0.01
10	8	G12NUT	1/2" HDG HEAVY 2H HEX NUT	0.07
11	4	X-UB1212	1/2" X 2-1/2" X 4-1/2" X 2" GALV. U-BOLT	2.65
11	4	X-UB1300	1/2" X 3" X 5" X 2" GALV U-BOLT	0.74
TOTAL WT. #				51.93

TOLERANCE NOTES

TOLERANCES ON DIMENSIONS, UNLESS OTHERWISE NOTED ARE:
 SAWED, SHEARED AND GAS CUT EDGES ($\pm 0.030"$)
 DRILLED AND GAS CUT HOLES ($\pm 0.030"$) - NO CONING OF HOLES
 LASER CUT EDGES AND HOLES ($\pm 0.010"$) - NO CONING OF HOLES
 BENDS ARE $\pm 1/2$ DEGREE
 ALL OTHER MACHINING ($\pm 0.030"$)
 ALL OTHER ASSEMBLY ($\pm 0.060"$)

PROPRIETARY NOTE:
 ALL DIMENSIONS AND TOLERANCES CONTAINED IN THIS DRAWING ARE PROPRIETARY INFORMATION OF VALMONT INDUSTRIES AND CONSIDERED A TRADE SECRET. ANY USE OR DISCLOSURE WITHOUT THE CONSENT OF VALMONT INDUSTRIES IS STRICTLY PROHIBITED.

DESCRIPTION
 SLIDER BRACKET
 WALL MOUNT
 2-3/8" & 2-7/8" O.D. PIPE

CPD NO.	DRAWN BY	ENG. APPROVAL
81	BMC	3/8/2011
CLASS	SUB	CHECKED BY
81	01	RCH
CUSTOMER		3/14/2011

Locations:
 New York, NY
 Atlanta, GA
 Los Angeles, CA
 Plymouth, IN
 Houston, TX
 Dallas, TX

Engineering
 Support Team:
 1-888-753-7446

SITE PRO 1
 A valmont COMPANY

PART NO.	SBWM
DWG. NO.	SBWM